



**WALLACE INFRASTRUCTURE DESIGN PTY LTD**

*Prepared for*

**Commercial 7 Pty Ltd  
ATF Commercial 7 Unit Trust**

**34 Wyndella Road LOCHINVAR NSW 2321**

**DA Stormwater Management Strategy**

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## List of Acronyms

AEP	Annual Exceedance Probability
AHD	Australian Height Datum
AR&R	Australian Rainfall and Runoff, Commonwealth of Australia, 2019
BASIX	Building Sustainability Index
BOM	Bureau of Meteorology
CC	Construction Certificate
DA	Development Application
EY	Exceedance per Year
FFL	Finished Floor Level
GPT	Gross Pollutant Trap
IFD	Intensity Frequency Duration
I.L	Invert Level
LGA	Local Government Area
MCC	Maitland City Council
NRAR	National Resources Access Regulator
PET	Potential Evapotranspiration
PSD	Permissible Site Discharge
R.L	Reduced Level
TWL	Top Water Level
WDG	Wallace Design Group



# 1. INTRODUCTION

## 1.1. Background

Wallace Design Group has been engaged by Commercial 7 Pty Ltd ATF Commercial 7 Unit Trust to formulate a Stormwater Management Strategy to support the Development Application for a 182 lot Manufactured Home Estate located at 34 Wyndella Road, Lochinvar NSW 2321. The site is situated within the Maitland City Council Local Government Area. The stormwater drainage strategy will address both stormwater quantity and quality requirements for the proposed development in accordance with ARR 2019, *Maitland City Council Development Control Plan* and *Manual of Engineering Standards (MOES)*.

A locality plan is shown in Figure 1.



Figure 1: Site Locality Plan

## 1.2. Site Description

The site is 10.74 ha of land located at 34 Wyndella Road, Lochinvar NSW and is generally bound by rural residential land to the north, south, east and west. The land is zoned as rural residential and is suitable for the proposed land use.

The site is bisected by three tributaries, herein referred to as the western, middle, and eastern tributaries. All three tributaries have no defined beds or banks within the mapped hydro lines. The existing western tributary drains to and under Wyndella Road immediately south of the proposed entry drive, whereas the middle and eastern catchments drain towards the south



across Lots 223 and 224, DP246447 ultimately crossing Wyndella Road near its intersection with the New England Highway.

The development application is proposing to divert all three tributaries and their upper catchments to Wyndella Road. The Western catchment will maintain its existing flow path through the culverts immediately south of the proposed access drive; whereas the middle and eastern catchments will be diverted with a bypass line and overland flow path to Wyndella Road and carried through a pit and pipe network to the lower crossing near the Wyndella Road intersection to the New England Highway (refer to the Wyndell Road Upgrade plans prepared by WDG, dated May 2025, Appendix C)

### 1.3. Proposed Development

The development proposes a 182 lot Manufactured Home Estate with additional community amenities including a central community centre, bowling lawn, pool and pickle ball court. A copy of the development layout plan is shown in Appendix A.

Access to the development will be maintained off Wyndella Road to the West which is proposed to be upgraded with this development and in conjunction with the neighbouring development to the west. Part road construction of the eastern side of Wyndella Road will consist of a 7m wide thoroughfare with kerb & gutter and a 1.5m wide footpath along the eastern side of the road corridor. Immediately in front of the site will be a BAR for access into the proposed development. The Wyndella Road Upgrade plans are further detailed in Appendix B.

The main entry road into the site is a divided road with the ingress and egress both maintaining 5m widths. The internal road network consists of north-south and east-west crossing roads in generally a grid formation as can be seen on the overall development plan. The internal road network is further detailed in the Civil and Stormwater Management Plan for the development (refer to Appendix B)

The upstream stormwater catchment which currently drain through the development site will have the stormwater diverted around, through and to Wyndella Road to remove all discharge from draining through the downstream property and towards the legal point of discharge (e.g. Wyndella Road).

The upstream stormwater catchment, currently bisecting the site within the existing stormwater crossings will be redirected through the site. The redirected network is further detailed in the Stormwater Management Plan.

The proposed development stormwater will be controlled for both quality and quantity through a stormwater pit and pipe network, GPT's and Stormwater Filters, and on-site detention basins. Rainwater reuse tanks, although not specifically modelled in MUSIC or DRAINS, will be provided for all residential lots in accordance with BASIX and Council guidelines at the time of construction and as part of the Construction Certificate stage of development.

Stormwater quality controls are to comply with *Maitland City Council – Manual of Engineering Standards*. Review of Council's guidelines indicate that stormwater quality controls are to have a target percentage reduction over the post-developed water quality outputs. To assess the development for water quality, a MUSIC water quality model with the proposed development being cleansed by Atlan Stormwater Filters is required. These are further detailed in Section 3.1.



Stormwater quantity controls are to comply with Maitland City Council – Manual of Engineering Standards and Maitland City Council's DCP. Design is to be in accordance with current ARR 2019 methodologies (ref. ARR 2019 Guidelines, online version). The design presented within this report has assessed all required output nodes for each type of use in a manner that maintain developed flow rates to the existing stormwater flow regimes.

#### **1.4. Scope of Work**

This Strategy has been undertaken to provide the following information in support of the Development Application:

- Identification of stormwater quality and quantity control requirements within Maitland City Council.
- Identify the impacts of the proposed development on existing waterways.
- Develop a strategy to minimise the stormwater discharge effects of the development on downstream properties/waterways.
- Provide concept design of the proposed stormwater management facilities in accordance with the adopted strategy.

#### **1.5. Available Data**

The following available information was utilised in the preparation of this report:

- A development masterplan supplied by BDA Architecture. A copy of the development layout plan is shown in Appendix A.
- Detailed Survey supplied by Commercial 7 Pty Ltd ATF Commercial 7 Unit Trust, as prepared by de Witt Consulting Surveyors, dated August 2023, Ref 14576.

#### **1.6. Strategy Objectives / Criteria**

##### **1.6.1. Stormwater Runoff Quality Criteria**

Stormwater runoff from the development should be treated prior to discharge, consistent with normal practice for new developments, and with consideration to opportunities for integration with existing site features and topography.

The methodology for Stormwater Runoff Quality typically involves selection of stormwater quality treatment devices based on identified opportunities for stormwater quality management referencing the development site and catchment conditions, and normal best practice.

Stormwater quality management for the subject site could comprise of a series of treatment train measures consisting of:

- Rainwater harvesting / reuse for reducing runoff volumes;
- Biofiltration, bioretention, sedimentation, or filtration basins
- Grassed swales;
- Water quality control ponds; and/or



- Gross pollutant traps (GPT's), or Proprietary water quality improvement devices for primary, secondary and/or tertiary treatment.

### **1.6.2. Stormwater Runoff Quantity Criteria**

The development shall consider stormwater quantity controls in accordance with Council's DCP and *Manual of Engineering Standards* which stipulates that developed stormwater flowrates shall not exceed the pre-developed flow rates for all storms up to and including the 1% Annual Exceedance Probabilities (AEP). The stormwater quantity controls will be conceptually sized and designed using DRAINS stormwater modelling software, the Initial-Loss Continuing Loss (IL-CL) design methodology, the guidelines and methodologies set out in Book 9 of *Australia Rainfall and Runoff: A Guide to Flood Estimation, 2019* (see references).

Stormwater flow management criteria includes:

- The adoption of a minor and major flow considerations to the design of the stormwater flow management system in accordance with Council's stormwater drainage design guidelines.
- Conveyance of major flows through the site in a safe manner.
- Limiting the post-development discharge rates from the subject site to the pre-development discharge rates for all storm events up to and including the 1%AEP.

### **1.6.3. Flooding Criteria**

Developable lot areas should have sufficient buildable area for a dwelling to be located at or above the flood planning level, which is the 1% AEP flood level plus 500mm freeboard for residential developments.

Review of the Maitland City Council's Public GIS Online Mapping System indicates the development is not within a flood studied area and therefore has no defined Flood Planning Level or PMF level.



## 2. STORMWATER MANAGEMENT STRATEGY

The proposed stormwater management strategy involves:

- The capture roof stormwater runoff, prior to discharge into the stormwater network, in a rainwater reuse tank in accordance with BASIX requirements for each building (expected to be 2500 to 3000L reuse tank). The reuse tanks have not been modelled as part of the development application; however, they are expected to contribute to the overall Stormwater Management Strategy.
- No additional provision for storage volume within the reuse tanks have been considered to offset on-site detention storage volume.
- Outlet controls designed to control stormwater flows for the minor and major AEP storm events in accordance with Council guidelines.
- Capture of stormwater from lots and road areas by a conventional pit and pipe drainage network located in the street and inter-allotment drainage easements.
- Diversion of the 4EY storm event through Atlan StormFilters for primary treatment of stormwater flows for reduction of TSS, TP and TN loadings to meet Council's stormwater quality targets.
- Diversion of stormwater flows to on-site detention basins for control of post-developed stormwater flows to pre-developed stormwater flow conditions in accordance with Council guidelines.

The modelling for the conceptual plan provided with the Development Application considers a holistic approach and looks at single source nodes for stormwater quality and quantity (e.g single nodes representing larger catchment areas). *The stormwater quality and quantity plan detailed in the Civil and Stormwater Plans as part of the DA Package is considered conceptual in nature and subject to change with the detailed design to be undertaken at Construction Certificate Stage, and as such, an updated model and report will be provided with the design at the time of Construction Certificate application.*



### 3. METHODOLOGY

#### 3.1. Stormwater Runoff Quality

The methodology for Stormwater Runoff Quality typically involves selection of stormwater quality treatment devices based on identified opportunities for stormwater quality management referencing the development site and catchment conditions, and normal best practice.

The performance of the stormwater management plan was undertaken using the MUSIC-X stormwater water quality modelling software. MUSIC X is a continuous simulation water quality model. The pollutants considered in the water quality modelling were total suspended solids (TSS), total phosphorus (TP), total nitrogen (TN), and Gross Pollutants (GP) which are typical components of stormwater runoff.

MUSIC input parameters include:

- Rainfall and potential evapotranspiration data.
- Catchment area and percentage impervious.
- Hydrologic parameters.
- Statistical pollutant generation parameters

MUSIC outputs include:

- Average annual pollutant export loads.
- Treatment train effectiveness, expressed in terms of pollutant reduction.

Input parameters used for modelling were derived from local data taken from Maitland Airport (Bureau of Meteorology Station Number: 61428), eWater's BOM Climate Dataset, parameter values in the *MUSIC User Manual* (version 6.1), and *Using MUSIC in Sydney's Drinking Water Catchment, A Sydney Catchment Authority Standard* (Published by Sydney Catchment Authority, Penrith, December 2012).

##### 3.1.1. MUSIC Parameters

###### Land Use Types

The post-developed land use was modelled using the following MUSIC source nodes:

- Urban Residential (Combined Roof & Lots, 85% impervious)
- Urban Mixed Use (Communal Facility, 100% impervious)
- Urban – Sealed Road (Paved, 100% Impervious)
- Urban – Revegetated Land (Grass, 100% Pervious)

The pollutant generation characteristics of the surface types are shown in Table 2 below.



## Rainfall and Evapotranspiration

The rainfall data and monthly average areal potential evapotranspiration (PET) values used for the modelling were obtained from data sourced from the Bureau of Meteorology Station Number: 61428 (Maitland Airport AWS) templated meteorological MUSIC file.

## Time Step

The MUSIC model was run with a time step of 6 minutes, as contained within the Maitland Airport Rainfall datasets.

## Hydrology

The MUSIC hydrology parameters for each land use type are summarised in Table 1. The individual land use nodes, rainfall thresholds, pervious area properties, and groundwater parameters were determined using Clay Loam *Using MUSIC in the Sydney Drinking Water Catchment, 2019*.

**Table 1: MUSIC Rainfall-Runoff Parameters**

Parameter	Site Land Use			
	Commercial	Revegetated Lands	Roof/Lots	Sealed Roads
<i>Impervious Area Properties</i>				
Rainfall Threshold (mm/day)	1.5	1.0	1.0	1.5
<i>Pervious Area Properties</i>				
Soil Storage Capacity (mm)	119	119	119	119
Initial Storage (% of Capacity)	25	25	25	25
Field Capacity (mm)	99	99	99	99
Infiltration Capac. Coeff. – a	180	180	180	180
Infiltration Capac. Exp. – b	3.0	3.0	3.0	3.0
<i>Groundwater Properties</i>				
Initial Depth (mm)	10	10	10	10
Daily Recharge Rate (%)	25	25	25	25
Daily Baseflow Rate (%)	25	25	25	25
Daily Deep Seepage Rate (%)	0	0	0	0

## Event Mean Concentrations

The pollutant baseflow and stormflow concentration parameters for the existing state and proposed land use types were determined from *Using MUSIC in Sydney's Drinking Water Catchment, Tables 4.6 and 4.7 (See references)* based on Rural Residential and Forest land uses. The adopted pollutant parameters for the specified land use types are provided in Table 2.



**Table 2: Adopted Land Use Baseflow and Stormflow Concentration Parameters**

Land Use and Flow Type	Total Suspended Solids (TSS) (log <sub>10</sub> mg/L)		Total Phosphorus (TP) (log <sub>10</sub> mg/L)		Total Nitrogen (TN) (log <sub>10</sub> mg/L)	
	Mean	Std. Dev	Mean	Std. Dev	Mean	Std. Dev
<i>Baseflow</i>						
Lot	1.2	0.17	-0.85	0.19	0.11	0.12
Commercial	1.2	0.17	-0.85	0.19	0.11	0.12
Road	1.20	0.17	-0.85	0.19	-0.11	0.12
Revegetated	1.15	0.17	-1.22	0.19	-0.05	0.12
Forest	0.9	0.13	-1.50	0.13	-0.14	0.13
<i>Stormflow</i>						
Residential	2.15	0.32	-0.60	0.25	0.30	0.19
Road	2.43	0.32	-0.30	0.25	0.34	0.19
Revegetated	1.95	0.32	-0.66	0.25	0.30	0.19
Commercial	2.15	0.32	-0.60	0.25	0.30	0.19

NOTE: Roofs have no base flow value, per *Using MUSIC in the Sydney Drinking Water Catchment*, 2019

### 3.2. Stormwater Runoff Quantity

Pre- and post-development stormwater flowrates are estimated using hydrological computer modelling software to demonstrate the magnitude of the local catchment flowrates in accordance with ARR 2019 and Council guidelines.

The post-development discharge is compared to the post-developed discharge without water quality controls, and if higher, detention is warranted in accordance with Council's standard requirements.

#### 3.2.1. Stormwater Flow Model

A stormwater flow model was prepared for the purpose of estimating pre- and post-development flowrates for the concept plan to demonstrate the magnitude of stormwater flows of the local catchments for the 20% (minor), 5% (intermediate) and 1% AEP (major) using both the ensemble and individual storm methodologies as outlined in ARR 2019.

##### 3.2.1.1. Catchment Plan and Model Data

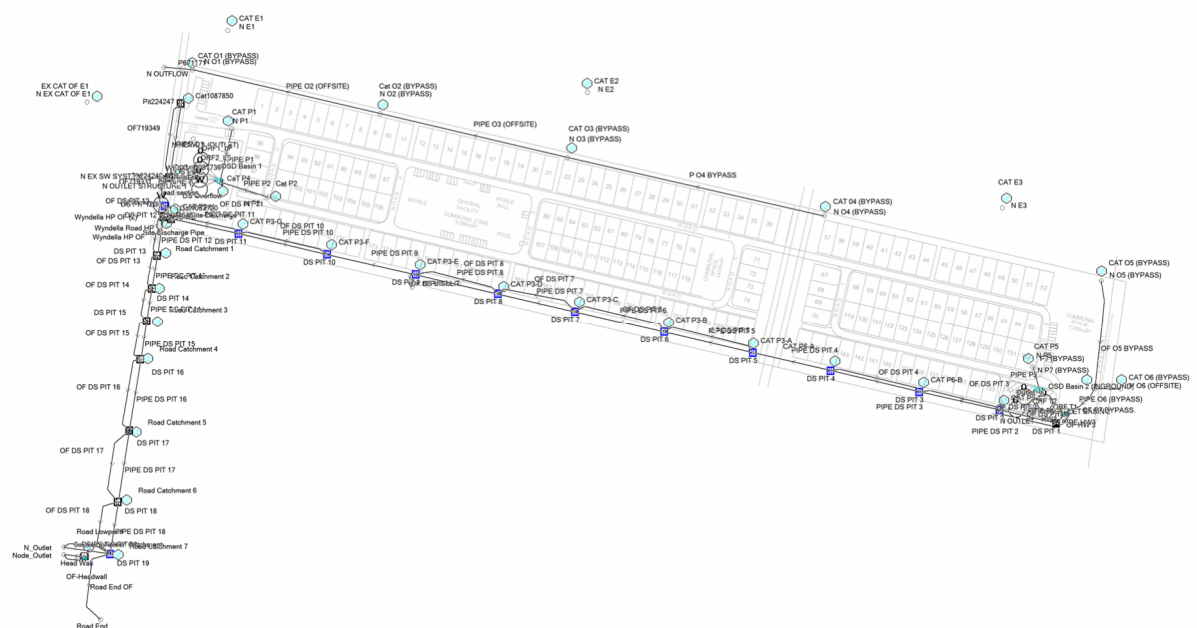
The surface runoff flowrates from the development were modelled using DRAINS hydrological modelling software for the pre- and post-developed state. The Initial Loss/Continuing Loss (IL/CL) Methodology for analysing stormwater flows was utilised to estimate the stormwater flowrates with the IL and CL values being determined based on the hierarchical approach as provided in ARR 2019 for developments in NSW.



The pre- and post- developed stormwater flowrates were analysed at two points of discharge from the site which are located at the southwest (Basin No.1) and southeast (OSD Tank No. 2) corner of the development where the existing tributaries discharge from the site. The outlet control structures for each are design to control to the pre-development condition; however, the stormwater discharge from Basin No. 1 and OSD Tank No. 2, along with the upstream stormwater flows will be diverted to and discharge to Wyndella Road as the legal point of discharge. The bypass lines are designed to cater for the 1% AEP with 50% blockage, and no overflow to the southern lots.

The internal catchments were subdivided in two main sub-catchment areas. These areas consist of developed areas (Catchments P1 to P4) draining to two GPT and Atlan Filter Systems and an on-site detention basin (Basin No. 1) located on the western boundary; and, the remaining developed area (Catchment 5), draining to a third Atlan Filter System and on-site detention Tank (refer to Appendix B). The remaining proposed on-site Catchments P6 and P7 cannot be captured in the system and will bypass the on-site detention tanks.

On-site detention was analysed using the above-described concept, and it is noted that a more detailed catchment plan and stormwater system analysis for the development will be completed at the time of Construction Certificate application (for each stage) to further detail the stormwater pit and pipe network, rainwater reuse tanks, and on-site detention basins associated with each stage of development. The conceptual DRAINs node layout is shown in Figure 2.



**Figure 2: DRAINs Node Layout**

The catchments and stormwater networks were modelled using the following base assumptions obtained from our research, Council information, and other on-line data (e.g. BOM website, ARR website, etc.):

- Model based on overall concept catchment areas and does not consider individual catchment areas for each stage of development or its pit and pipe network.



- Per above point, the model excluded the 50% blockage factor on all sag pits, and 20% blockage factor on all on-grade pits for major storm events. The model will be amended at Construction Certificate stage to include this criteria.
- A Rainwater Reuse Tank (RWT's) will be provided for each house as required by BASIX. An additional storage volume within the RWT's may be considered at the time of Construction Certificate application; however, *for the conceptual design, separate storage within the rainwater reuse tank was not considered.*
- On-site detention control for the minor, intermediate, and major storm events in accordance with Council Guidelines.

The methodology for stormwater quantity comprised quantitative analysis of available data to estimate pre- and post-developed stormwater flow behaviours from the development site. The analysis involved examination of surface hydrology to assess runoff characteristics from the site and sizing of stormwater mitigation devices to negate the impact of the development on existing flowrates.

This involved the following steps:

- Estimation of pre- and post-developed peak stormwater flowrates at the downstream drainage outlets of the site using the DRAINS software modelling package.
- The critical storm was then selected for each AEP from the 20% to 1% AEP, based on the peak discharge from the site for each storm event. The hydrographs of these 'critical' storms were plotted to determine the approximate volume of storage required.
- Identification of potential locations for on-site stormwater detention structures to reduce post-development discharge flowrates and revise the developed model in DRAINS to include the detention and outlet structures required to reduce post-development discharge flowrates.

### 3.2.1.2. Rainfall Data

Rainfall data for the 12EY to 1% AEP storm events were obtained through the BOM website (e.g. ARR Data HUB) for storm durations of 10 minutes to 4.5 hours for downloading directly into the DRAINS modelling software. These data sets were produced using the following geographical data points:

Location	Lochinvar, NSW
Latitude	= 32.696 S
Longitude	= 151.465 E

### 3.2.1.3. DRAINS IL/CL Model Parameters

Table 3 summarises the initial loss and continuing loss data sets adopted for use in DRAINS. These values were assessed utilising the 5-step hierarchal approach as outlined in ARR 2019. Based on our research regarding steps 1 through 4 of this process, no data existing that would appropriately represent this site; therefore, Step 5 was assumed appropriate for this development. In NSW the adopted CL value is to be multiplied by 0.4; therefore the adopted CL value has been modified from the value provided on the BOM website. This reduction provides an additional factor of safety for the design values adopted.



**Table 3: Adopted Initial Loss-Continuing Loss Values**

Parameter	Adopted Values
Impervious Area Initial Loss (mm)	1
Impervious Area Continuing Loss (mm)	0
Pervious Area Initial Loss (mm)	18
Pervious Area Continuing Loss (mm)	2.0 (0.8 adopted)

NOTE: Value for CL multiplied by 0.4 in accordance with ARR 2019.

### 3.2.1.4. Model Sub-Catchment Data

Surface roughness values,  $n^*$ , used in the DRAINS models are summarised in Table 4.

**Table 4: Roughness parameter values,  $n^*$ , adopted in the DRAINS models**

Model - surface type	Surface roughness ' $n^*$ ' value
Forest Channel	0.15
Short prairie grass	0.10 to 0.21
Pervious (grassed) areas	0.21
Impervious (paved) areas	0.011

Sub-catchment pervious and impervious area percentage values used in the DRAINS modelling are summarised in Table 5 below.

**Table 5: Area Percentage values adopted in the DRAINS models**

Model - type	Area Percentage	
	Impervious	Pervious
Existing Catchments (Existing Site)		
E1, E2	0	100
E3	0	100
Existing Catchments - Bypass (Developed Site)		
O1, O2, O3, O4, O5	0	100
O6	0	100
Proposed Catchments (Controlled)		
P1, P2, P5	65	35
P3, P4, P6	0	100
Proposed Catchments (Controlled)		
P7 (located on-site)	0	100



## 4. MODEL RESULTS

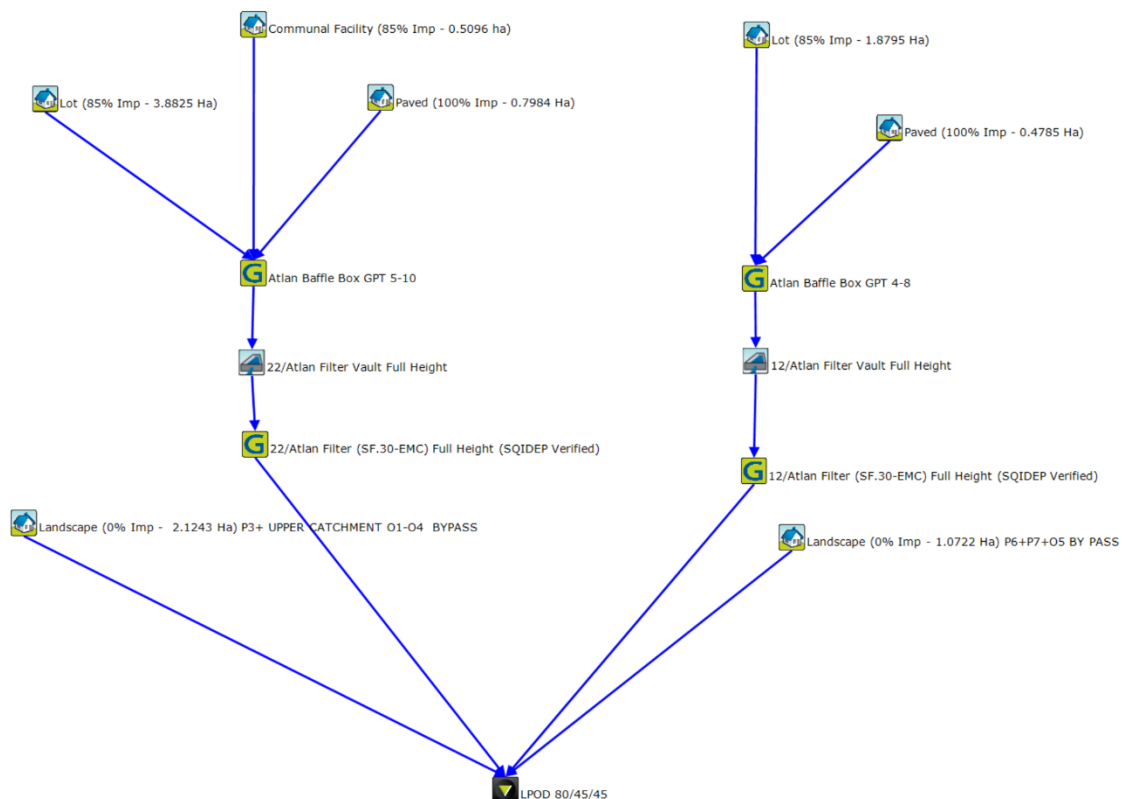
### 4.1. Stormwater Runoff Quality

#### 4.1.1. MUSIC Results – Post Development Land Use (No Treatment)

The modelled average annual pollutant loads leaving the site in its post development land use state, without any treatment measures, is shown in Table 6. Pollutant load estimates are provided for total suspended solids (TSS), total phosphorus (TP) and total nitrogen (TN). Figure 3 shows the node layout used in the MUSIC modelling.

**Table 6: MUSIC Results for the Site's Post Development Land Use (No Treatment)**

Land Use	Average Annual Pollutant Load (kg/yr)		
	Total Suspended Solids (TSS)	Total Phosphorus (TP)	Total Nitrogen (TN)
Post Development	10,480	16.9	111.0



**Figure 3: MUSIC Node Layout**

#### 4.1.2. MUSIC Results – Post Development Land Use (with Treatment)

The MUSIC model results for the post development land use, with treatment measures, is documented below, enabling the evaluation of the treatment train effectiveness. In assessing treatment devices used for the development, we assessed a series of treatment train measures



to minimise the environmental impact of the development. The assessment included the following measures:

- Rainwater harvesting / reuse for reducing runoff volumes;
- Biofiltration, bioretention, sedimentation, or filtration basins
- Water quality control ponds; and/or
- Gross pollutant traps (GPT's), or Proprietary water quality improvement devices for primary, secondary and/or tertiary treatment:

The measures chosen will minimise the impact of the development on the adjacent waterways.

#### **4.1.2.1. Treatment Devices**

Treatment devices modelled in MUSIC for the treatment of runoff from the developments impervious surface areas include:

- Rainwater Tanks (considered, but not included in the modelling)
- Atlan Storm Filter System; and,
- GPT's.
- Swales (not modelled as part of the concept, but may be considered at CC stage) for areas that cannot be capture into the proposed treatment measures).

##### **4.1.2.1.1. Rainwater Tanks**

The rainwater tank node, *although not specifically modelled* will form a part of the Stormwater Management Strategy. The MUSIC model will be updated at Construction Certificate Stage as part of the overall design strategy to include a RWT node immediately downstream of the roof area node, using the default rainwater tank treatment node parameters within MUSIC. The rainwater tank node will generally include the following node data for each lot:

- Rainwater tank volume of 2.2kL, or as required by BASIX, per each residential lot.
- Daily and annual water usage demand based on Section 4.6.7, Non-potable water demands for rainwater tanks, *Using MUSIC in Sydney's Drinking Water Catchment, 2019*.

##### **4.1.2.1.2. Atlan Baffle Box GPT**

The initial treatment devices are two Atlan Baffle Box GPT structures. The GPT is a proprietary structure for the removal of Gross Pollutants and the GPT nodes were included as the first node in the treatment train immediately upstream of the Atlan StormFilter System. There are two GPT's for the development which are summarised in Table 7 below.



**Table 7: Atlan Baffle Box Parameters**

Parameter	GPT Structure	
	GPT 4-8	GPT 5-10
<u>Inlet Properties</u>		
Low Flow By-pass (m <sup>3</sup> /s)	0.0	0.0
High Flow Bypass (m <sup>3</sup> /s), (4EY Peak Storm)	0.140	0.274
<u>Target Pollutant Removal Properties</u>		
TSS	80	80
TP	30	30
TN	10	10
Gross Pollutants	100	100

#### 4.1.2.1.3. Atlan StormFilter System

The secondary treatment for the development is the Atlan StormFilter system. The Atlan StormFilter System is a proprietary structure for the primary removal of TSS, TP and TN and is included as the second treatment node in the treatment train prior to discharge into on-site detention basins. There are two node inputs required to analyse the Atlan StormFilter System which are the Filter Vault (Full Height) and the SF.30 EMC Filters. These parameters are provided in Table 8 and Table 9.

**Table 8: Atlan Filter Vault Full Height Treatment Parameters**

Parameter	Filter Vaults	
	12/Atlan	22/Atlan
<u>Inlet Properties</u>		
Low Flow By-pass (m <sup>3</sup> /s)	0	0.0
High Flow Bypass (m <sup>3</sup> /s)	100	100
<u>Storage Properties</u>		
Extended Detention Depth (m)	12	22
Surface Area (m <sup>2</sup> )	0.85	0.85
Exfiltration Rate (mm/hr)	0	0
Evaporative Loss as % of PET (%)	0	0
<u>Outlet Properties</u>		
Low Flow Pipe Diameter (mm)	137	185
Overflow Weir Width (m)	4	6
Submerged Zone with Carbon Present	0.07	0.071



**Table 9: Atlan Filter (SF.30-EMC) Parameters**

Parameter	Filter (SF.30-EMC)	
<i><u>Inlet Properties</u></i>	12/Atlan	22/Atlan
Low Flow By-pass (m <sup>3</sup> /s)	0.0	0.0
High Flow Bypass (m <sup>3</sup> /s)	0.036	0.066
<i><u>Target Pollutant Removal Properties</u></i>		
TSS	85	85
TP	74	74
TN	59	59
Gross Pollutants	0	0

#### 4.1.2.2. Modelling Results

The modelled average annual pollutant loads leaving the site in its post development land use, utilising treatment measures, is shown in Table 10. Pollutant load estimates are provided for total suspended solids (TSS), total phosphorus (TP) and total nitrogen (TN). Figure 3 shows the node layout used in the MUSIC modelling.

**Table 10: MUSIC Results for the Site's Post Development Land Use (with Treatment)**

Land Use	Average Annual Pollutant Load (kg/yr)		
	Total Suspended Solids (TSS)	Total Phosphorus (TP)	Total Nitrogen (TN)
Post Development	605	3.65	44.5

A direct comparison between the existing state and post development pollutant loads with treatment generated by the modelled sources, is summarised in Table 11.

**Table 11: MUSIC Model Treatment Train Effectiveness Results**

Pollutant	Export Value		Treatment Train Effectiveness (% Reduction)
	Pre Development	Post Development with treatment measures	
TSS (kg/yr)	10,480	605	94.2%
TP (kg/yr)	16.9	3.65	78.4%
TN (kg/yr)	111	44.5	59.9%
Gross Pollutants	1364	0	100%

The treatment train effectiveness results above indicate that the pollutant reduction performance provides for a Neutral or Beneficial Effect in accordance with the requirements of *MidCoast Council's WSD Guidelines*, Oct. 2019 and as requested in the pre-development application minutes.



## 4.2. Stormwater Runoff Quantity

### 4.2.1. DRAINS Model Results

Following determination of the pre-developed peak flow rates, the post-developed DRAINS model was run for various design storm durations and stormwater drainage configurations to assess the preliminary conveyance sizing required to convey the 20% to 1% AEP through the site and for assessment of stormwater detention requirements.

The model-predicted stormwater flowrates for the pre- and post-development conditions were assessed using both the ensemble and individual storm methodologies. Within the DRAINS modelling software, the peak ensemble storm for the pre- and post-development storm was initially assessed to determine the peak median storm for each design storm event. Using output generated from this assessment preliminary sizing of the on-site detention basins was undertaken. The model-predicted stormwater flowrates were further assessed using the preliminary basin sizing for the pre- and post-development conditions.

The on-site detention basin's model predicted flowrates for the pre- and post-developed state with detention using the ensemble peak flow storms and storm bursts for On-site Detention Basin No. 1 and Tank No. 2 are compared and summarised in Table 12 and Table 13, respectively.

**Table 12: DRAINS Ensemble Peak Flow Results with Detention Basin No. 1**

AEP (%)	Existing State		Developed State		% Change
	Peak Flowrate (m <sup>3</sup> /s)	Critical Storm Duration/Burst (mins, burst)	Peak Flowrate (m <sup>3</sup> /s)	Critical Storm Duration/Burst (mins, burst)	
20	0.450	25, 4	0.426	45, 9	-5.3
5	0.720	20, 8	0.577	60, 6	-19.8
1	1.148	15, 8	0.925	45, 6	-19.4

**Table 13: DRAINS Ensemble Peak Flow Results with Detention Tank No. 2**

AEP (%)	Existing State		Developed State		% Change
	Peak Flowrate (m <sup>3</sup> /s)	Critical Storm Duration/Burst (mins, burst)	Peak Flowrate (m <sup>3</sup> /s)	Critical Storm Duration/Burst (mins, burst)	
20	1.142	30, 8	1.07	45, 10	-6.3
5	1.910	25, 6	1.77	30, 8	-7.3
1	2.989	20, 10	2.83	20, 10	-5.3

### 4.2.2. DRAINS Modelled Detention Basin and Outlet Controls

The conceptual design considers two (2) on-site detention systems (one above ground basin and one in-ground basin) with pre-treatment measures undertaken before stormwater enters the Basin 1 or 2. Table 14 shows a summary of the concept outlet control(s) used in the



DRAINS modelling for the proposed development. The values are subject to change with the detailed Construction Certificate Design.

**Table 14: Summary of Basin and Outlet Structures**

Basin	Parameter	Description
1	Basin Storage Volume 1%AEP	2290m <sup>3</sup>
	Outlet Structure, Minor Flow Bypass	2-350mm, I.L 39.75
	Outlet Structure, Intermediate & Major Flow Bypass	425mm Weir
	Outlet Structure, Top (set at 1%AEP level)	2.4m weir w/grated top, I.L 41.45
	Emergency Overflow Structure	3.0m weir, R.L 41.5
	Outlet Pipe from Basin	750mm RCP
2	Basin Storage Volume	1035m <sup>3</sup>
	Outlet Structure, Minor Flow Bypass	1-250mm Orifice, I.L 55.4
	Outlet Structure, Major Flow Bypass	1-250mm Orifice, I.L 56.6
	Emergency Overflow Structure	1.8m Weir at R.L 57.7
	Outlet Pipe from Tank	450mm RCP

NOTE: The Stormwater Management Strategy does not include any individual lot on-site OSD tanks to ensure the development can maintain controls without individual lot development controls



## 5. SOIL AND WATER MANAGEMENT DURING CONSTRUCTION

Soil and water management devices to minimise land disturbance during the development construction phase are to be provided in accordance with the publication *Managing Urban Stormwater: Soils and Construction* (Landcom, 2004).

### STAGE REQUIREMENTS

Prior to each stage of works, and as part of the detailed design for each stage, a Staged Erosion and Sedimentation Control Plan will be prepared illustrating all controls to be implemented for that Stage of works.

### PREPARATION OF EROSION AND SEDIMENTATION CONTROLS

A detailed erosion and sedimentation control plan for each stage of works is to be undertaken during the detailed design stage of the proposed development. The erosion and sedimentation control plan should generally contain the following range of management practices for effective soil and water management during a land disturbance phase:

- Minimise the area of soil disturbed and exposed to erosion by phasing works so that land disturbance is confined to minimum areas.
- Erect barrier fencing to minimise disturbance by preventing vehicular and pedestrian access to restricted areas.
- Limit access for plant etc. to current construction area to limit amount of disturbed area.
- Conserve topsoil for site rehabilitation/revegetation when site works are complete.
- Installation of sediment filters, such as silt fences, straw bales, or turf strips downstream of disturbed areas.
- Control water flow from the top of, and through the development area. In particular, divert upslope runoff around works and limit slope length to 80 metres on disturbed lands if rainfall is expected.
- Where appropriate, reduce the effects of wind erosion by controlling on-site traffic movement and watering bare soil areas.
- Provision of shaker humps / pads near construction entry and exit locations to remove excess soil materials from vehicle tyres and underbodies.
- Rehabilitate disturbed lands quickly.
- Ensure that all erosion and sediment control measures are kept in a properly functioning condition until all site disturbance works are completed and the site is rehabilitated.

The erosion and sedimentation controls are the minimum requirements for all construction activity, and where deemed necessary, may be modified by the certifying authority at the time of construction.



## 6. SUMMARY AND CONCLUSIONS

### **At Source Management**

Although a BASIX's requirements review is not a specific requirement of this stormwater management strategy, it is anticipated that BASIX's requirements would require all individual dwellings to provide rainwater tanks for re-use in conjunction with other BASIX's requirements and Council guidelines. Where installed, rainwater tanks provide at-source stormwater management benefits.

### **Stormwater Flow Management (stormwater runoff conveyance and quality)**

The strategy for management of stormwater runoff from the development is depicted Appendix B, and comprises:

- Capture of stormwater from lot and road reserve areas by a conventional pit and pipe drainage network located in the street or in inter-allotment drainage easements, where required.
- Conveyance of captured stormwater within the pit and pipe network to a GPT for primary treatment, Atlan StormFilter system for secondary treatment, and to the on-site detention basin(s) for tertiary controls.

MUSIC modelling has demonstrated that the proposed treatment devices will treat developed stormwater runoff to meet the requirements of Council's DCP, and on this basis, it is considered that no further water quality controls will be required within the proposed development. The individual lots will be required to provide Reuse Tanks in accordance with BASIXs and Council Guidelines; however, they do not form a part of the water quality modelling for the purpose of approvals.

Details of the proposed local drainage pipe, water quality and on-site detention network will be detailed at the time of Construction Certificate application to Council's standard and specifications.

As illustrated by Appendix B there is sufficient area within the developments footprint to provide stormwater drainage management measures to negate the impact of the proposed development.



## 7. REFERENCES

- Maitland City Council Development Control Plan
- Maitland City Council *Manual of Engineering Standard (MOES)*.
- Ball J, Babister M, Nathan R, Weeks W, Weinmann E, Retallick M, Testoni I, (Editors) *Australian Rainfall and Runoff: A Guide to Flood Estimation*, ©Commonwealth of Australia (Geoscience Australia), 2019 (available online through [arr.ga.gov.au](http://arr.ga.gov.au)).
- Coombes, P., and Roso, S (Editors), *2019 Runoff in Urban Areas, Book 9 in Australian Rainfall and Runoff – A Guide to Flood Estimation*, © Commonwealth of Australia (Geoscience Australia), 2019 (available online through [arr.ga.gov.au](http://arr.ga.gov.au)).
- Australian Rainfall and Runoff Data Hub ([data.arr-software.org](http://data.arr-software.org))
- Australian Runoff Quality, Engineers Australia, 2006.
- *Using MUSIC in Sydney's Drinking Water Catchment*, Published by WaterNSW, Parramatta, June 2019





# **Appendix A**

## Development Layout Plan



LEGEND

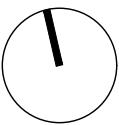
<div></div>	Site Area	10.75ha
<div></div>	Bushfire Buffer (10m - 13m)	1.75ha
<div></div>	Home Sites	5.40ha
	Private Roads	1.27ha
<div></div>	Communal Landscape Areas	3.76ha
<div></div>	Caravan Parking	17 Bays
<div></div>	Visitor Parking (6.1 x 2.5m) MINIMUM DISABLE SPACE (DIS)	47 Bays
<div></div>	Storm water	0.31 ha
<div></div>	HYDRANT	
<div></div>	Stage 1 screen planting (refer to landscape plans for detail)	

DEVELOPMENT SUMMARY

Home Type			
<div></div>	10 x 23.75/27.75m	20	12%
<div></div>	12 x 23.75/27.75m	162	88%
<b>TOTAL</b>		<b>182</b>	

Note:  
Corner lots are 1m wider to accommodate an additional side setback.

Communal Landscape Area = Area shown in green minus stormwater and approximate driveway area for each lot.







## **Appendix B**

### **Civil and Stormwater Management Plan**

Development Site & Wyndella Road Upgrade





# CIVIL & STORMWATER WORKS for 34 WYNDELLA ROAD LOCHIVAR NSW 2321

Prepared by  
**WALLACE INFRASTRUCTURE DESIGN PTY LTD**



LOCATION PLAN  
(NTS)

DRAWING NUMBER	DRAWING DESCRIPTION
C01.01	LEGEND, DRAWING SCHEDULE AND LOCATION PLAN
C02.01	GENERAL NOTES
C03.01	EROSION & SEDIMENTATION CONTROL PLAN
C03.02	EROSION & SEDIMENTATION CONTROL DETAILS
C04.01	GENERAL ARRANGEMENT PLAN
C04.02	BULK EARTHWORKS PLAN
C04.03	ROAD PLAN (1 OF 3)
C04.04	ROAD PLAN (2 OF 3)
C04.05	ROAD PLAN (3 OF 3)
C04.06	ROAD 1-8-2 AND 9 LONGITUDINAL SECTIONS (1 of 4)
C04.07	ROAD 1-8-2 AND 9 LONGITUDINAL SECTIONS (2 of 4)
C04.08	ROAD 1-8-2 AND 9 LONGITUDINAL SECTIONS (3 of 4)
C04.09	ROAD 1-8-2 AND 9 LONGITUDINAL SECTIONS (4 of 4)
C04.10	ROAD 6-2 AND 3 LONGITUDINAL SECTIONS (1 of 2)
C04.11	ROAD 6-2 AND 3 LONGITUDINAL SECTIONS (2 of 2)
C04.12	ROAD 4 AND 5 LONGITUDINAL SECTIONS
C04.13	NORTH-SOUTH ROAD AND VISITOR PARKING ACCESS DR. LONGITUDINAL SECTIONS
C05.01	EXISTING STORMWATER CATCHMENT PLAN
C05.02	DEVELOPED STORMWATER CATCHMENT PLAN
C05.03	STORM WATER MANAGEMENT PLAN (1 OF 3)
C05.04	STORM WATER MANAGEMENT PLAN (2 OF 3)
C05.05	STORM WATER MANAGEMENT PLAN (3 OF 3)
C06.01	SITE TYPICAL ROAD SECTIONS
C06.02	ON-SITE DETENTION BASIN No. 1 PLAN
C06.03	ON-SITE DETENTION BASIN No. 1 TYPICAL SECTIONS
C06.04	ON-SITE DETENTION TANK No. 2 PLAN
C06.05	ON-SITE DETENTION TANK No. 2 TYPICAL SECTIONS

## ABBREVIATIONS

A/G	ABOVE GROUND	IFC	ISSUED FOR CONSTRUCTION	SV	STOP VALVE
AGG	AGGREGATE	IFCR	ISSUED FOR CLIENT REVIEW	SW	STORMWATER
ARCH	ARCHITECT	IL	INVERT LEVEL	SWP	STORMWATER PIT
ASP	ASPHALT	IO	INSPECTION OPENING	THK	THICKNESS
BM	BENCHMARK	KIP	KERB INLET PIT	TJ	TRAVERSE JOINT
CAD	COMPUTER AIDED DRAFTING	L	LENGTH	TOW	TOP OF WALL
C-C	CENTRE TO CENTRE	LP	LAMP POST (SURVEY)	TW	TRADE WASTE
CH	CHAINAGE	MAX	MAXIMUM	TYP	TYPICAL
CI	CAST IRON	MC	MASS CONCRETE	U/G	UNDERGROUND
CJ	CONSTRUCTION JOINT	MH	MANHOLE	UNO	UNLESS NOTED OTHERWISE
CL	COVER LEVEL or CENTRE LINE	MIN	MINIMUM	U/S	UPSTREAM
CO	CLEAN OUT (SUBSOIL)	MISC	MISCELLANEOUS	VC	VITRIFIED CLAY PIPE
COG	CHANGE OF GRADE	N	NORTH	VP	VENT PIPE
CONC	CONCRETE	NRV	NON RETURN VALVE	VR	VERTICAL RISER
CTRL	CONTROL	NTS	NOT TO SCALE	W	WIDTH
CR	CROWN	OD	OUTER DIAMETER	WC	WATER CLOSET
CTS	CENTRES	OF	OVERFLOW	WL	WATER LEVEL
D	DEPTH	OH	OVERHEAD	WM	WATER METER
DIA	DIAMETER	PP	POWER POLE	WS	WASTE STACK
DP	DOWNPIPE	PPE	PERSONAL PROTECTIVE EQUIP.		
DRG	DRAWING	PVC	POLYVINYLCHLORIDE	UNITS	
D/S	DOWN STREAM	PV	PRESSURE VENT	mm	MILLIMETRES
DTM	DIGITAL TERRAIN MODEL	PVP	PRESSURE VENT PIPE	cm	CENTIMETRES
EB	EDGE BITUMIN	QA	QUALITY ANALYSIS	m	METRES
EX	EXISTING	QTY	QUANTITY	m <sup>2</sup>	SQUARE METRES
ESL	EXISTING SURFACE LEVEL	R	RADIUS	m <sup>3</sup>	CUBIC METRES
FH	FIRE HYDRANT	RC	REINFORCED CONCRETE	L/s	LITRES PER SECOND
FHR	FIRE HOSE REEL	REV	REVISION	ha	HECTARES
FFL	FINISHED FLOOR LEVEL	RL	REDUCED LEVEL		
FGL	FINISHED GROUND LEVEL	RW	RETAINING WALL		
FSL	FINISHED SURFACE LEVEL	SFW	SEALED FLOOR WASTE		
FW	FLOOR WASTE	SL	SURFACE LEVEL		
GA	GENERAL ARRANGEMENT	SMH	SEWER MANHOLE		
GI	GALVANISED IRON	SMV	SEWER MAIN VENT		
GFL	GARAGE FINISHED LEVEL	SOD	SIDE OUTLET DRAIN		
IC	INSPECTION CHAMBER	S/S	STAINLESS STEEL		
ID	INSIDE DIAMETER	STD	STANDARD		

## LINETYPES & SYMBOLS

### GENERAL

SITE BOUNDARY	---
CADASTRAL BOUNDARY	---
EASEMENT BOUNDARY	---
EXISTING FEATURES	
CONTOURS	---30---
STORMWATER PIPE	---
STORMWATER PIT	---
ELECTRICAL CABLE - U/G	---E---
ELECTRICAL CABLE - O/H	---OHE---
EXISTING SEWER PIPE	---S---
EXISTING WATER	---W---
EXISTING WATER (HYDRANT)	●
EXISTING WATER (STOP VALVE)	X
EXISTING TELECOM	---OFC---
COMMUNICATIONS PIT	---
EXISTING GAS	---G---
EXISTING BUILDING	---
TREES / SHRUBS	---
PROPOSED - BUILDING (INDICATIVE ONLY)	
DWELLING NUMBER	UNIT 01
FINISHED FLOOR LEVEL	FFL 5.00
BUILDING FOOTPRINT	---

### PROPOSED - UTILITIES

SEWER	---
WATER	---
COMMUNICATIONS	---
COMMUNICATIONS - OPTIC FIBRE	---OFC---
COMMUNICATIONS PIT	---
GAS	---
ELECTRICAL CABLE - U/G	---E---
ELECTRICAL CABLE - O/H	---OHE---
REDUNDANT	---X X X X X---
PROPOSED - EROSION AND SEDIMENT CONTROL	
SITE EXCLUSION FENCE	---
SEDIMENT FENCE	---
DIVERSION DRAIN	---
STABLISED SITE ACCESS	---
MATERIAL STOCKPILE	---
SLOPE DIRECTION	---
GEOTEXTILE INLET FILTER	---
MESH AND GRAVEL INLET FILTER	---
SANDBAG / HAY BALES	---
PROPOSED CONTOURS	
MAJOR CONTOUR INTERVAL	10
MINOR CONTOUR INTERVAL	---

### PROPOSED - STORMWATER

SW PIPE (INCL. DIA/GRADE/LENGTH)	ØXXXmm uPVC X.X% GRADE, X.XXm
SW PIPE - CHARGED (INCL. DIA/GRADE/LENGTH)	ØXXXmm uPVC CHARGE X.X% GRADE, X.XXm
SW PIT - GRATED / JUNCTION	---
SW - KERB INLET PIT (INCLUDING LINTEL)	---
RAINWATER/REUSE TANK	---
GROSS POLLUTANT TRAP	---
GRATED DRAIN	---
INFILTRATION TRENCH	---
HEADWALL	---
SUBSOIL	---
SW SWALE	---
PIPE RISER	XX
PIPE DROPPER	XX
OVERLAND FLOW PATH	---
SW PIT NUMBER	---
SW CATCHMENT (NUMBER / AREA IN ha)	XX XXXX

### PROPOSED - CIVIL

CONCRETE	---
ASPHALTIC CONCRETE	---
SAND	---
EARTH	---
RIP RAP	---
BUILDING / STRUCTURES	---
TIMBER	---
BLOCK PAVERS	---
RETAINING WALL	---
KERB RAMP	---
VEHICULAR CROSSING	---
FENCE	---
BATTER	---
BOLLARD - TYPE 1 (FIXED)	● B-T1
BOLLARD - TYPE 2 (REMOVABLE)	● B-T2
LINEMARKING - CHEVRON	---
EXPANSION JOINT	EJ
TRAVERSE JOINT	TJ
CONTROL JOINT	---CJ---
FINISHED GRADE LEVELS	25.758
EXISTING GRADE LEVELS	25.710

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D	LEC AMENDMENTS	01.05.25	K.B.	
C	COUNCIL AMENDMENTS - S34 OUTCOMES	28.11.24	K.B.	
B	COUNCIL AMENDMENTS - PREL S34	19.08.24	K.B.	
A	ISSUED FOR DA APPROVAL	02.02.24	K.B.	

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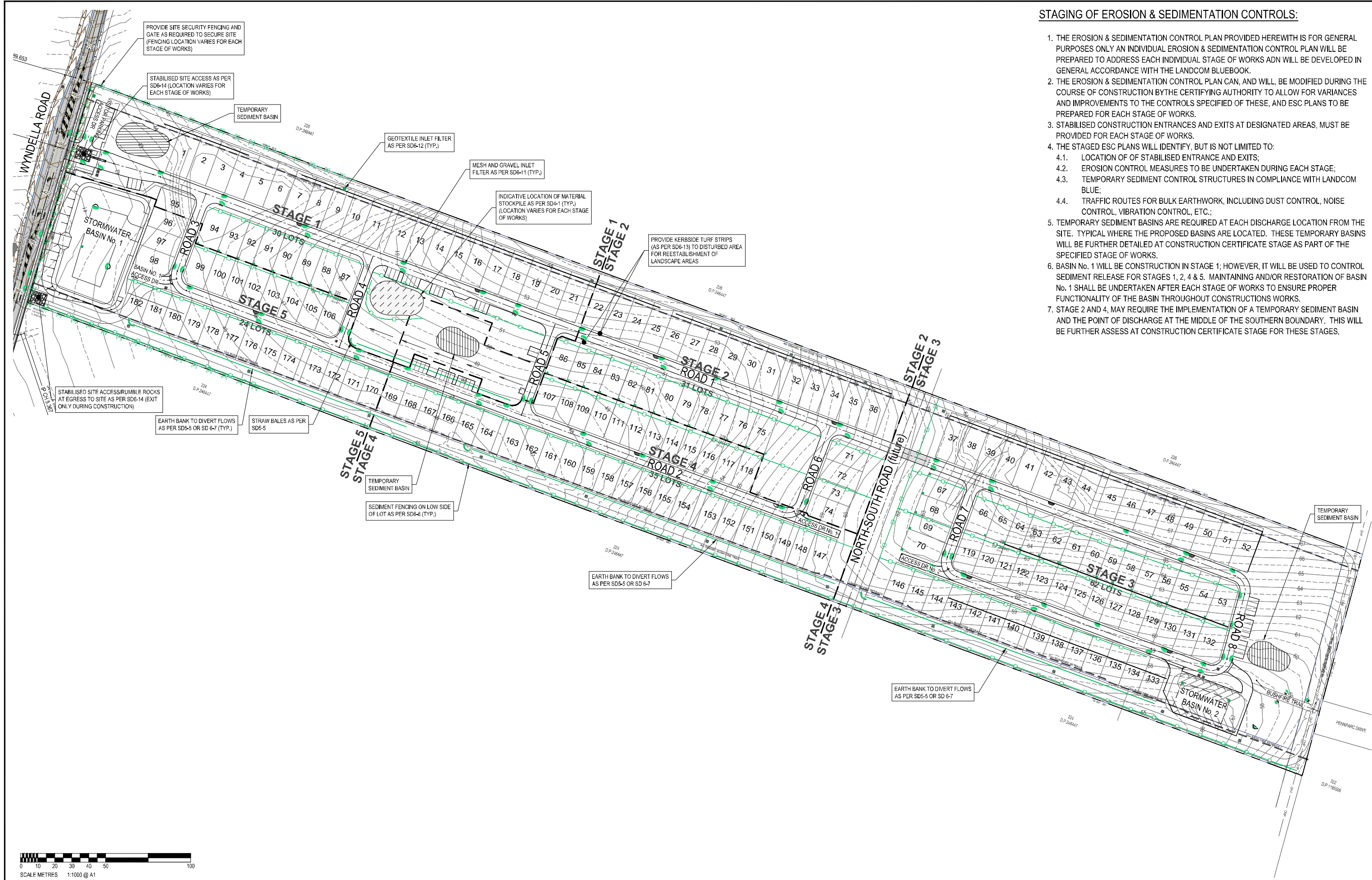
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PROJECT:	CIVIL WORKS for
	LOT 225, DP 246447
	34 WYNDELLA ROAD
	LOCHIVAR NSW 2321

DRAWING TITLE:	CIVIL SERVICES
	LEGEND, DRAWING SCHEDULE AND LOCATION PLAN

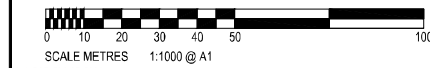
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MS	KB	AS	KB	-	01.05.25
PROJECT No.		DRAWING No.		REV	
23128		C01.01		D	





STAGING OF EROSION & SEDIMENTATION CONTROLS:

1. THE EROSION & SEDIMENTATION CONTROL PLAN PROVIDED HERewith IS FOR GENERAL PURPOSES ONLY AN INDIVIDUAL EROSION & SEDIMENTATION CONTROL PLAN WILL BE PREPARED TO ADDRESS EACH INDIVIDUAL STAGE OF WORKS AND WILL BE DEVELOPED IN GENERAL ACCORDANCE WITH THE LANDCOM BLUEBOOK.
2. THE EROSION & SEDIMENTATION CONTROL PLAN CAN, AND WILL, BE MODIFIED DURING THE COURSE OF CONSTRUCTION BY THE CERTIFYING AUTHORITY TO ALLOW FOR VARIANCES AND IMPROVEMENTS TO THE CONTROLS SPECIFIED OF THESE, AND ESC PLANS TO BE PREPARED FOR EACH STAGE OF WORKS.
3. STABILISED CONSTRUCTION ENTRANCES AND EXITS AT DESIGNATED AREAS, MUST BE PROVIDED FOR EACH STAGE OF WORKS.
4. THE STAGED ESC PLANS WILL IDENTIFY, BUT IS NOT LIMITED TO:
  - 4.1. LOCATION OF OF STABILISED ENTRANCE AND EXITS;
  - 4.2. EROSION CONTROL MEASURES TO BE UNDERTAKEN DURING EACH STAGE;
  - 4.3. TEMPORARY SEDIMENT CONTROL STRUCTURES IN COMPLIANCE WITH LANDCOM BLUE;
  - 4.4. TRAFFIC ROUTES FOR BULK EARTHWORK, INCLUDING DUST CONTROL, NOISE CONTROL, VIBRATION CONTROL, ETC.;
5. TEMPORARY SEDIMENT BASINS ARE REQUIRED AT EACH DISCHARGE LOCATION FROM THE SITE. TYPICAL WHERE THE PROPOSED BASINS ARE LOCATED. THESE TEMPORARY BASINS WILL BE FURTHER DETAILED AT CONSTRUCTION CERTIFICATE STAGE AS PART OF THE SPECIFIED STAGE OF WORKS.
6. BASIN No. 1 WILL BE CONSTRUCTION IN STAGE 1; HOWEVER, IT WILL BE USED TO CONTROL SEDIMENT RELEASE FOR STAGES 1, 2, 4 & 5. MAINTAINING AND/OR RESTORATION OF BASIN No. 1 SHALL BE UNDERTAKEN AFTER EACH STAGE OF WORKS TO ENSURE PROPER FUNCTIONALITY OF THE BASIN THROUGHOUT CONSTRUCTIONS WORKS.
7. STAGE 2 AND 4, MAY REQUIRE THE IMPLEMENTATION OF A TEMPORARY SEDIMENT BASIN AND THE POINT OF DISCHARGE AT THE MIDDLE OF THE SOUTHERN BOUNDARY. THIS WILL BE FURTHER ASSESS AT CONSTRUCTION CERTIFICATE STAGE FOR THESE STAGES.



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REV	DESCRIPTION	DATE	APPVD	END'D
D	LEC AMENDMENTS	01.05.25	K.B.	
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CIVIL WORKS for  
LOT 225, DP 246447  
34 WYNDELLA ROAD  
LOCHINVAR NSW 2321

DRAWING TITLE:  
**CIVIL SERVICES**  
EROSION AND SEDIMENTATION  
CONTROL PLAN

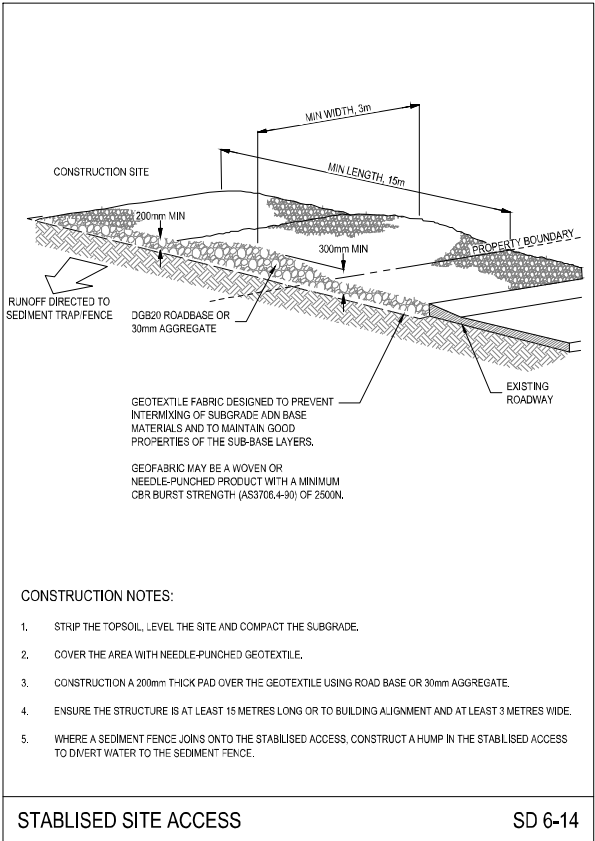
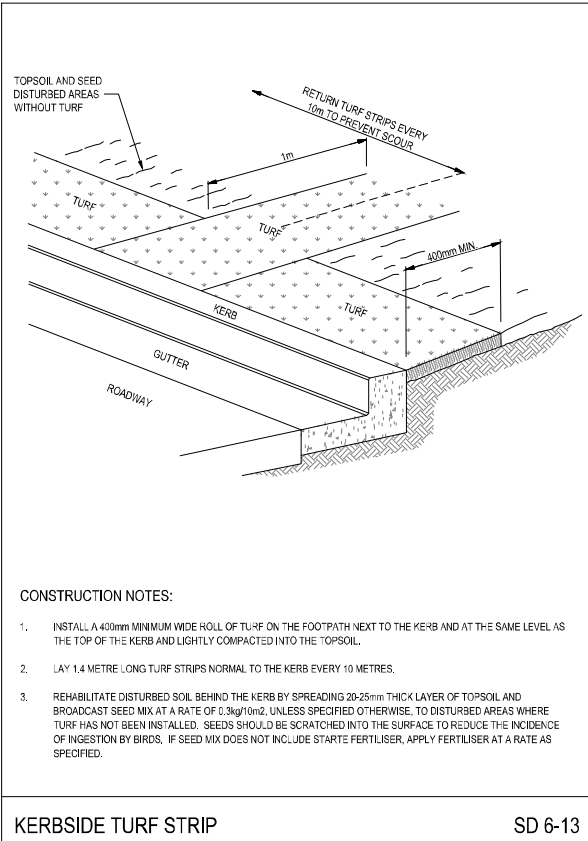
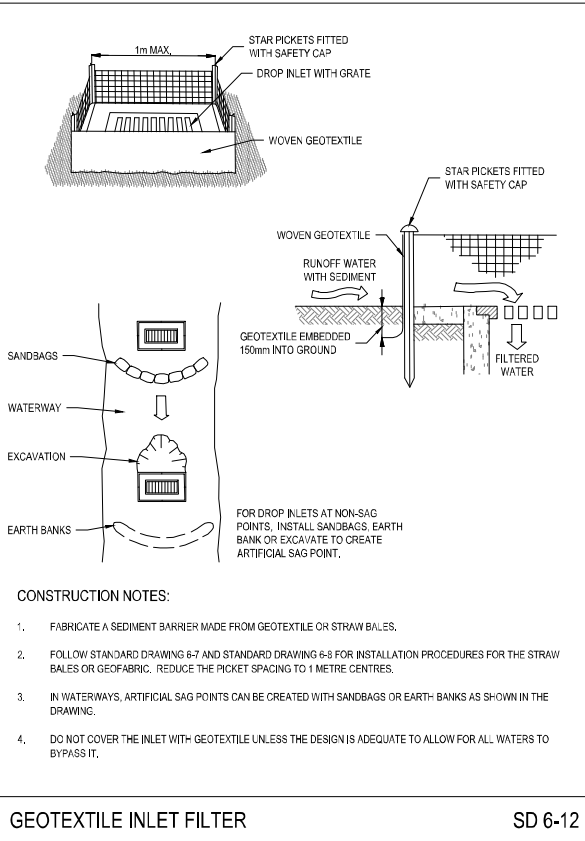
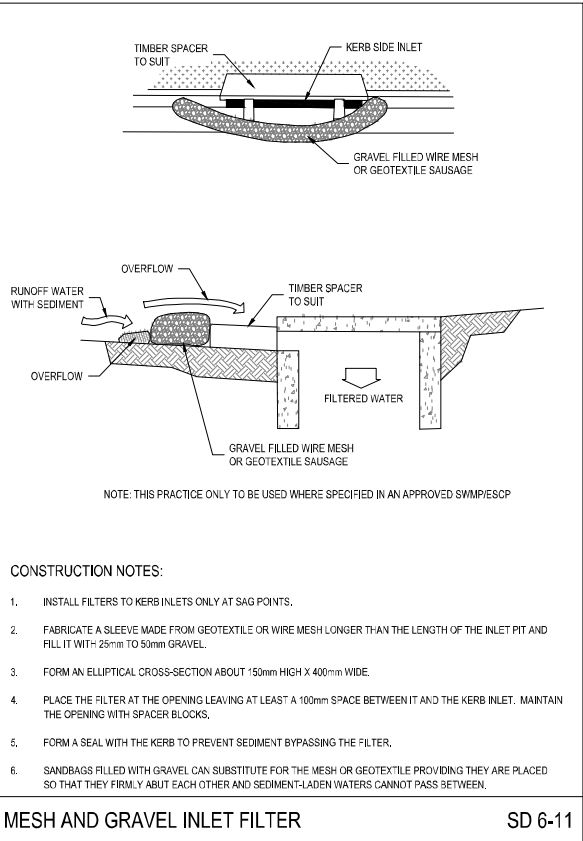
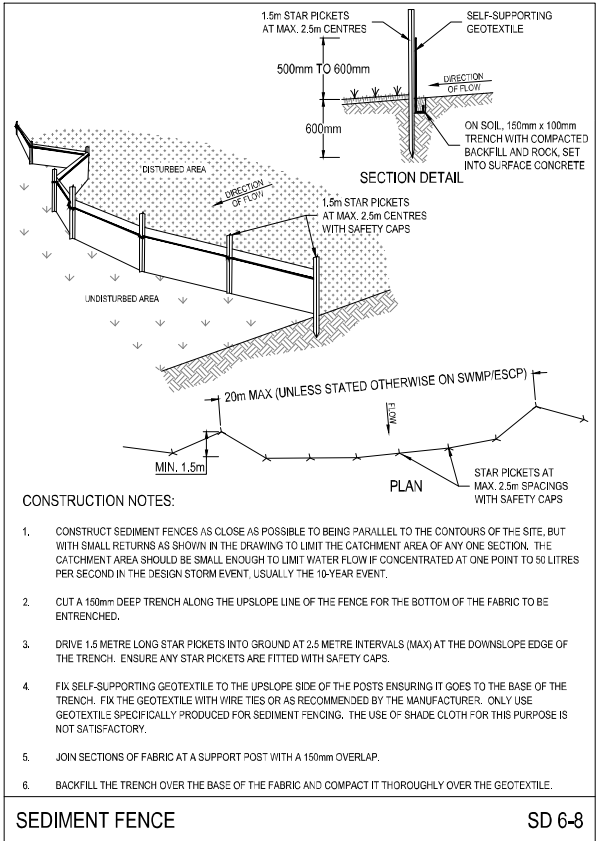
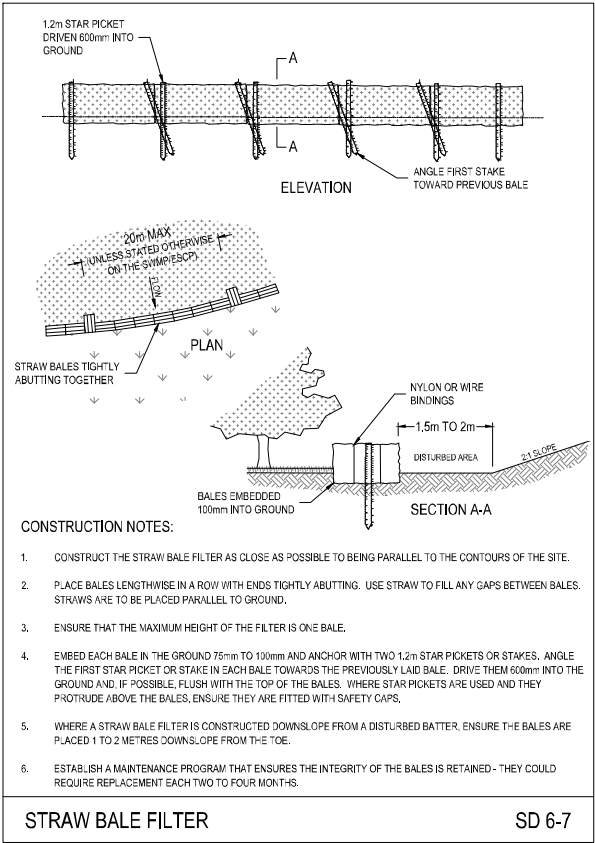
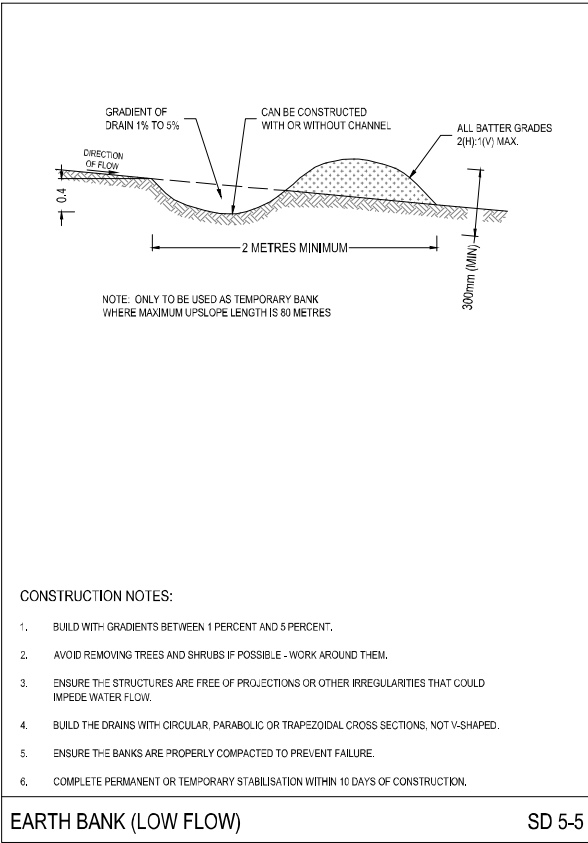
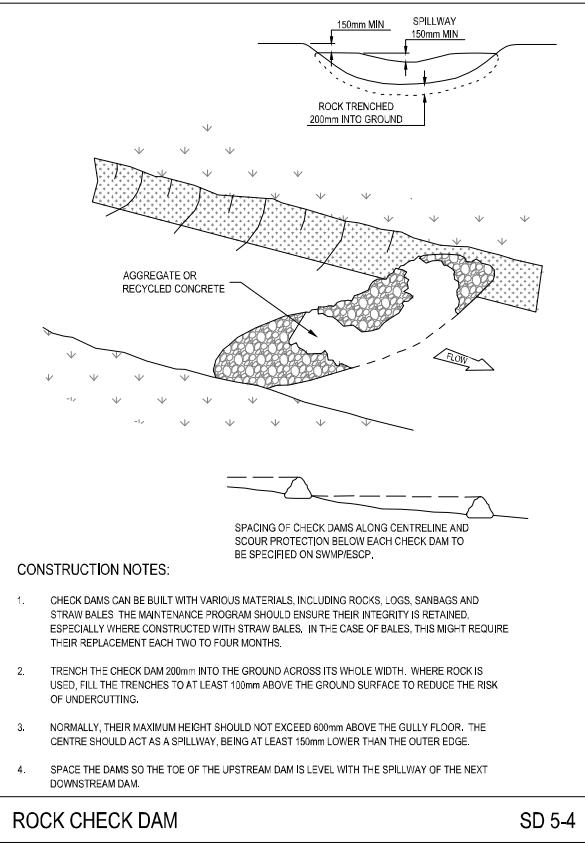
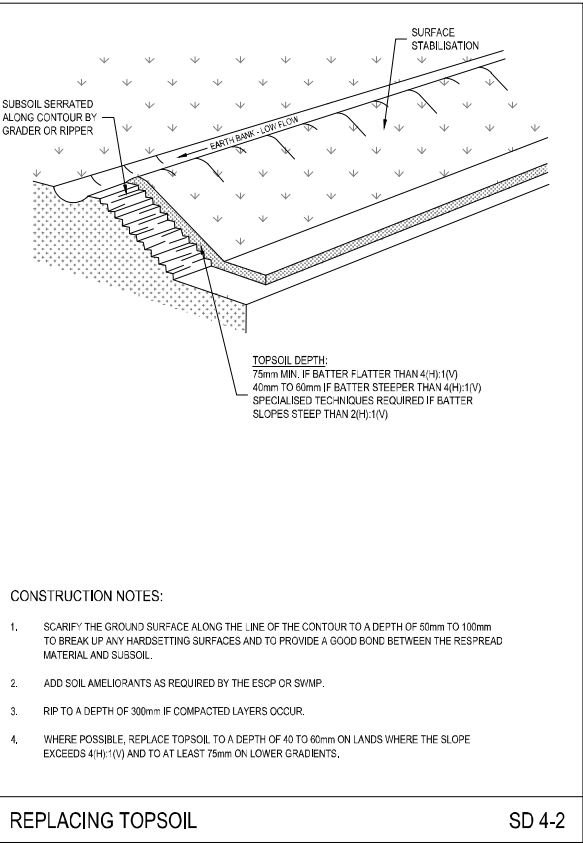
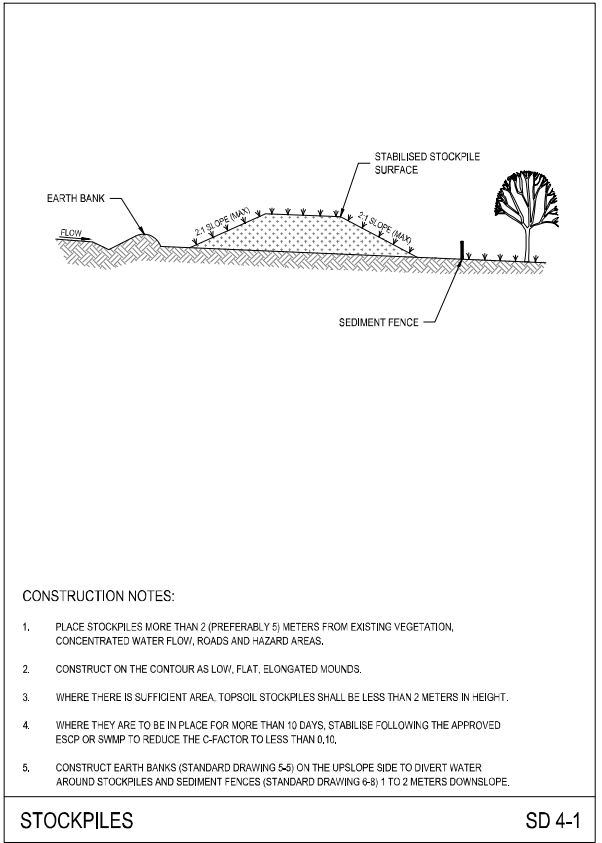
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MS	KB	AS	KB	-	01.05.25		
PROJECT No.				DRAWING No.		REV	
23128				C03.01		D	

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NOTE

1. ALL EROSION & SEDIMENTATION CONTROL (ESC) PLAN DETAILS SHOWN ARE FROM "MANAGING URBAN STORMWATER - SOILS AND CONSTRUCTION", LANDCOM, 2004. DETAILS, ALTHOUGH NOT SPECIFICALLY IDENTIFIED FOR USE ON THE ESC PLAN (SHEET C03.01), ARE PROVIDED IN THE EVENT ADDITIONAL CONTROLS ARE REQUIRED TO MINIMISE ON-SITE EROSION DURING THE COURSE OF CONSTRUCTION. IF AN ESC DETAIL IS NOT SHOWN, CONTRACTOR SHALL REFER TO THE ABOVE REFERENCED GUIDE.



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C	COUNCIL AMENDMENTS - S34 OUTCOMES	28.11.24	K.B.	
B	COUNCIL AMENDMENTS - PREL S34	19.08.24	K.B.	
A	ISSUED FOR DA APPROVAL	02.02.24	K.B.	

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HUNTERS HILL NSW 2110



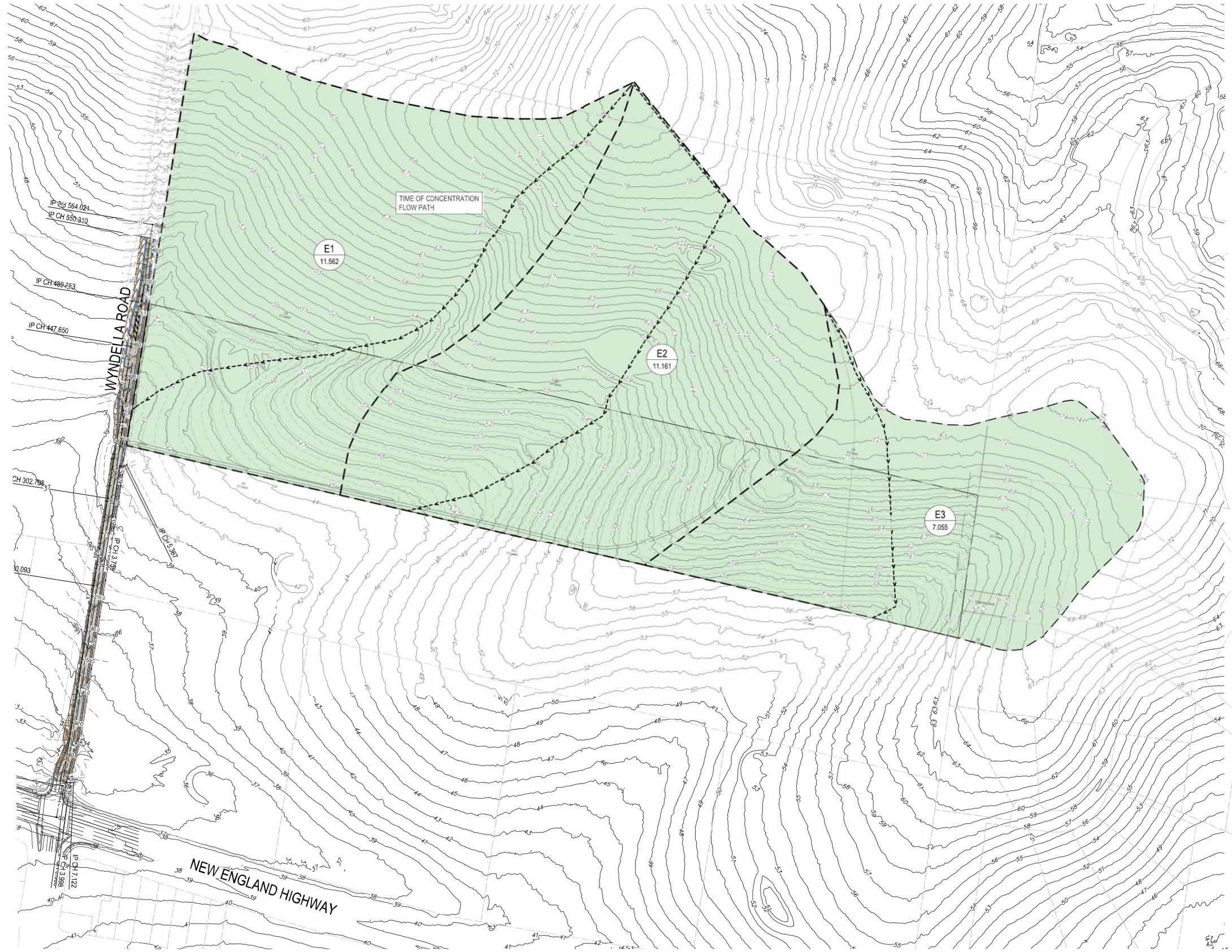
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PROJECT:  
CIVIL WORKS for  
LOT 225, DP 246447  
34 WYNDELLA ROAD  
LOCHINVAR NSW 2321

DRAWING TITLE:  
**CIVIL SERVICES**  
EROSION AND SEDIMENTATION  
CONTROL DETAILS

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DA APPROVAL NOT TO BE USED FOR CONSTRUCTION						
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PROJECT No. 23128			DRAWING No. C03.02			





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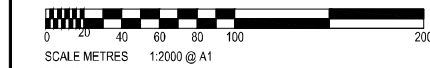
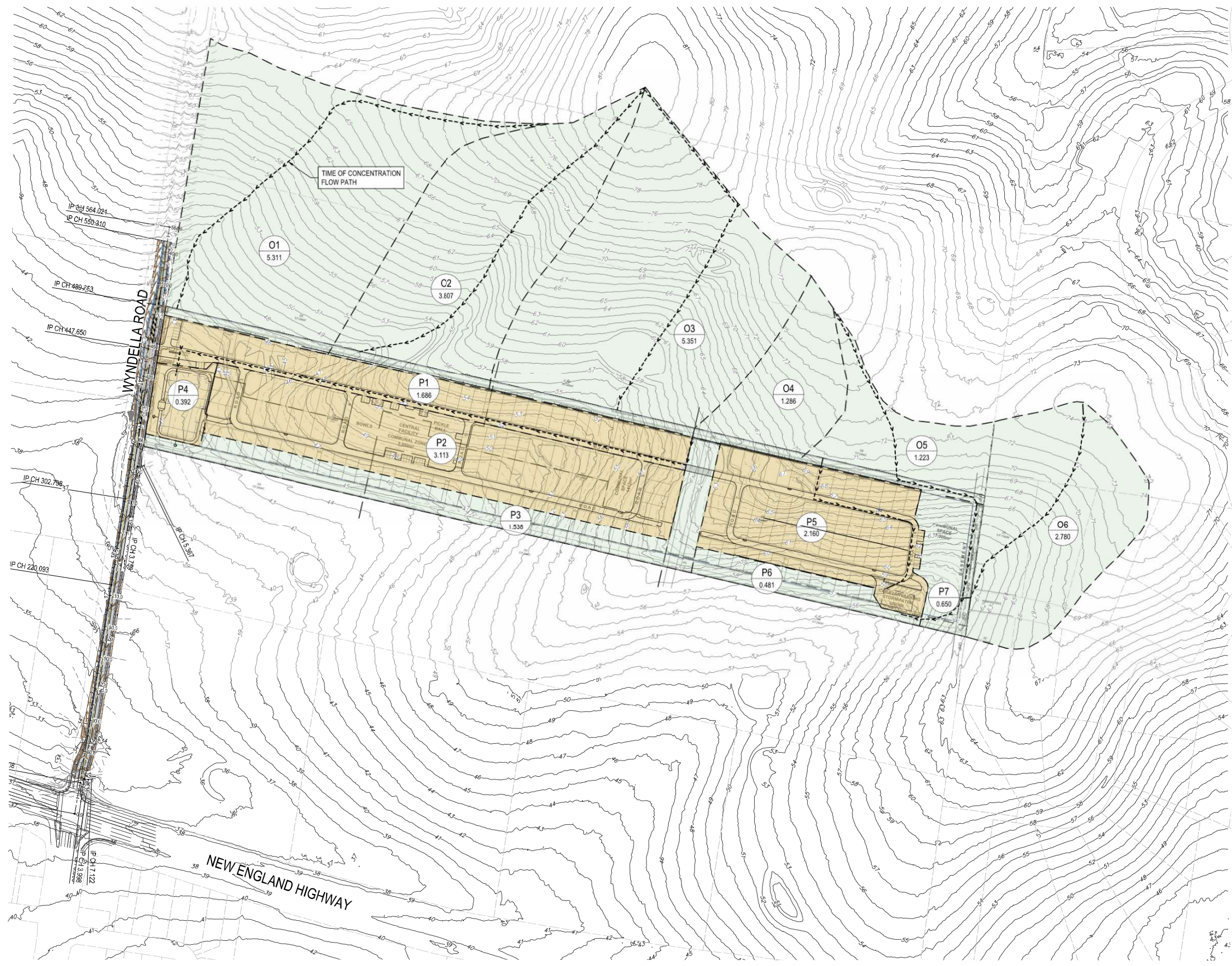
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PROJECT:  
CIVIL WORKS for  
LOT 225, DP 246447  
34 WYNDELLA ROAD  
LOCHINVAR NSW 2321

DRAWING TITLE:  
**CIVIL SERVICES**  
EXISTING STORMWATER  
CATCHMENT PLAN

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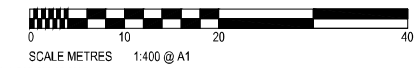
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LOT 225, DP 246447  
34 WYNDELLA ROAD  
LOCHINVAR NSW 2321

DRAWING TITLE:  
**CIVIL SERVICES**  
DEVELOPED STORMWATER  
CATCHMENT PLAN

DRAWING STATUS						
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PROJECT No. 23128		DRAWING No. C05.02			REV	D



1. TEMPORARY DRAINAGE SWALES TO BE DESIGNED AT CC STAGE TO ALLOW TEMPORARY FLOW PATHS TO ON-SITE DETENTION BASIN WHERE CATCHMENTS NEED CONNECTION TO FUTURE OUTLETS.



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PROJECT:  
CIVIL WORKS for  
LOT 225, DP 246447  
34 WYNDELLA ROAD  
LOCHINVAR NSW 2321

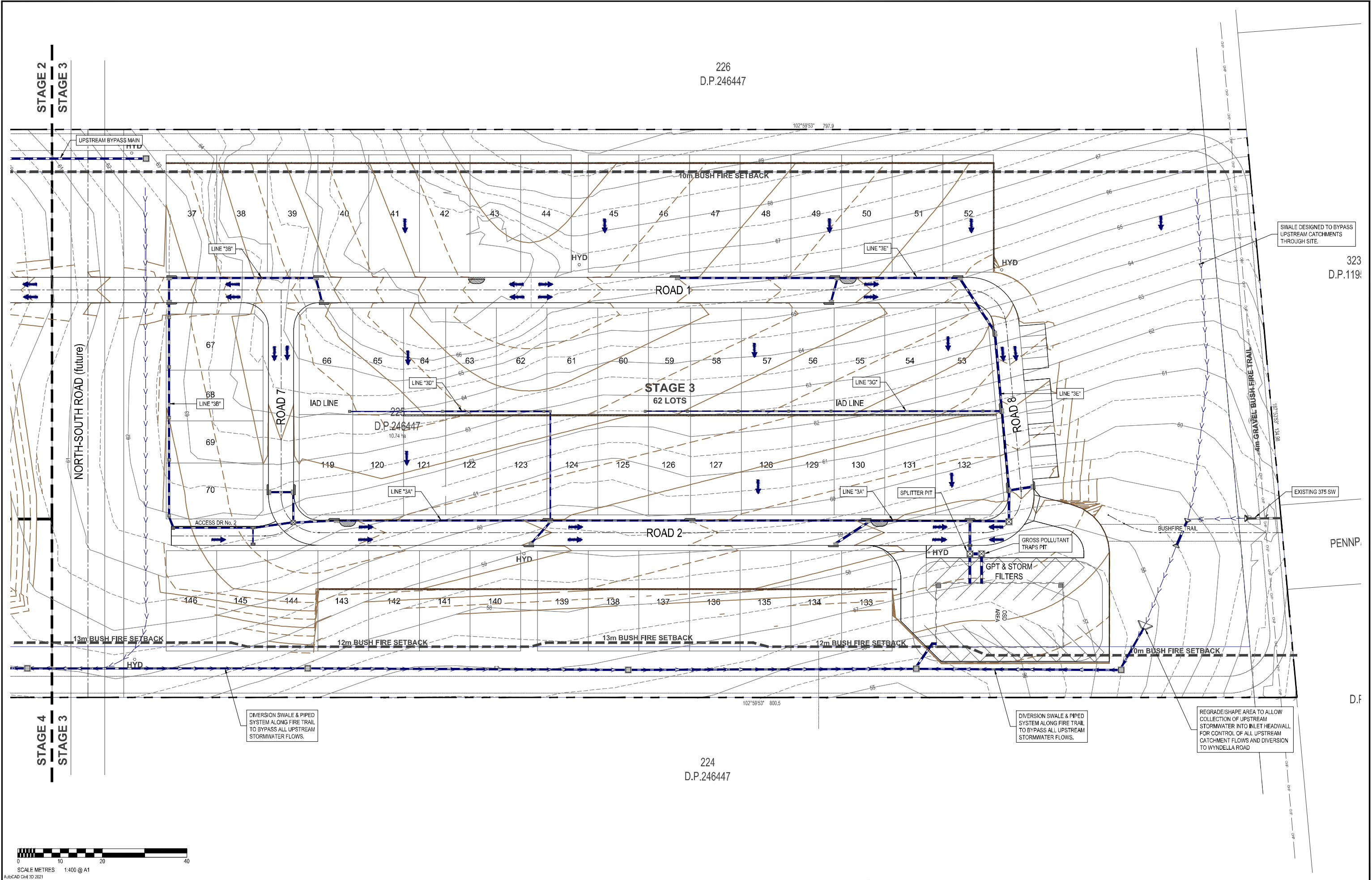
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STORMWATER MANAGEMENT  
PLAN (1 OF 3)


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23128		C05.03			D







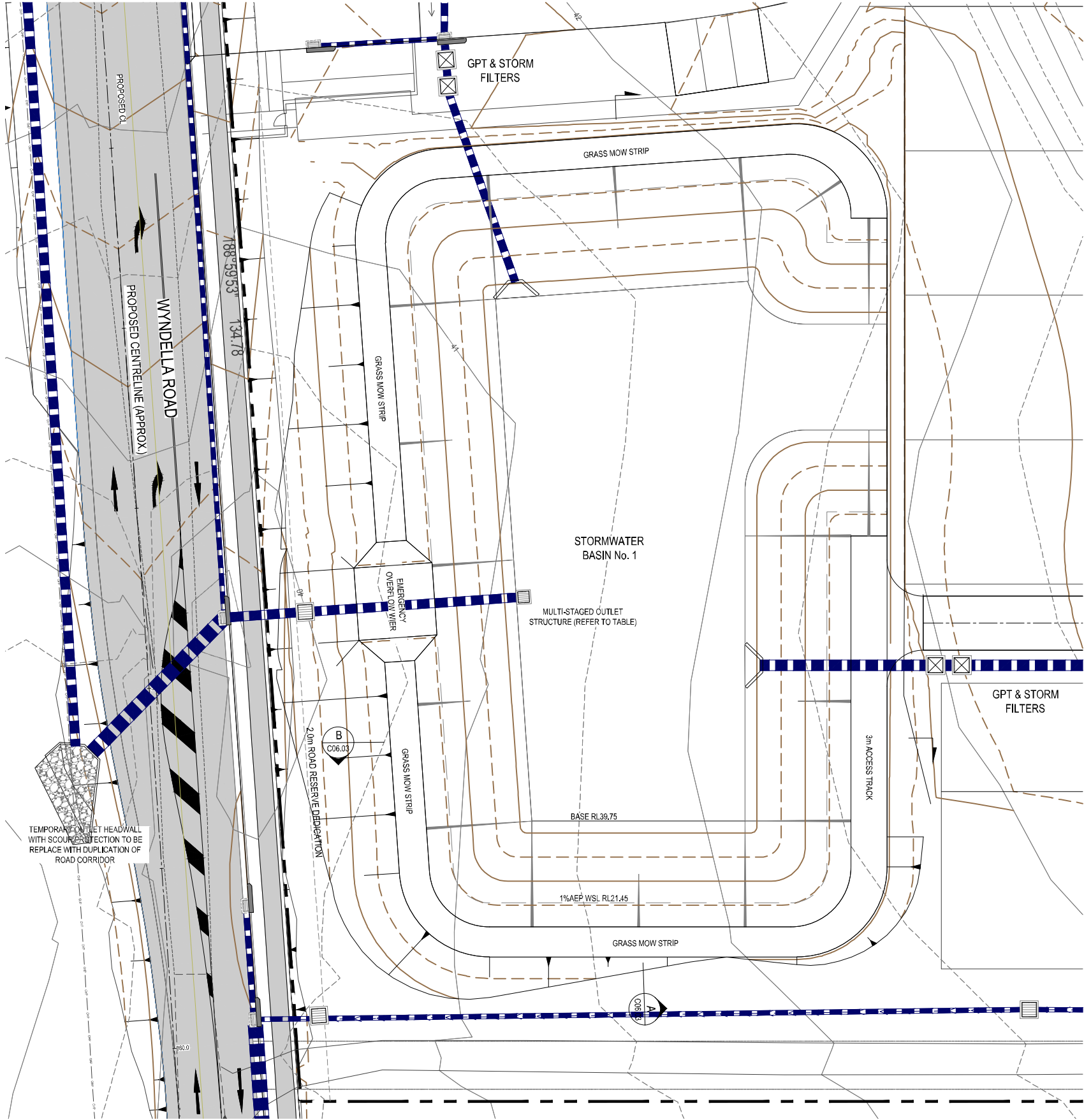


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PROJECT No. 23128		DRAWING No. C05.05		DATE: 01.05.25		REV: D							



BASIN AND OUTLET CONTROL STRUCTURE NOTES:

TOTAL BASIN STORAGE	2290 m <sup>3</sup>
WATER SURFACE LEVELS	
20% AEP	40.65 AHD
5% AEP	41.04 AHD
1% AEP	41.45 AHD
OUT CONTROL STRUCTURE	
ORIFICE 1 & 2	Ø350 AT 39.75 AHD
WEIR 1	425mm AT 40.65 AHD
WEIR 2	900X900 PIT AT 41.45 AHD
EMERGENCY SPILLWAY	3m AT 41.50 AHD

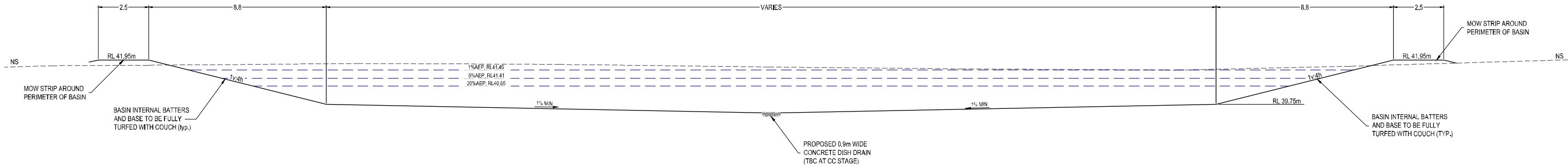


ON-SITE DETENTION BASIN No. 1 PLAN  
1:200

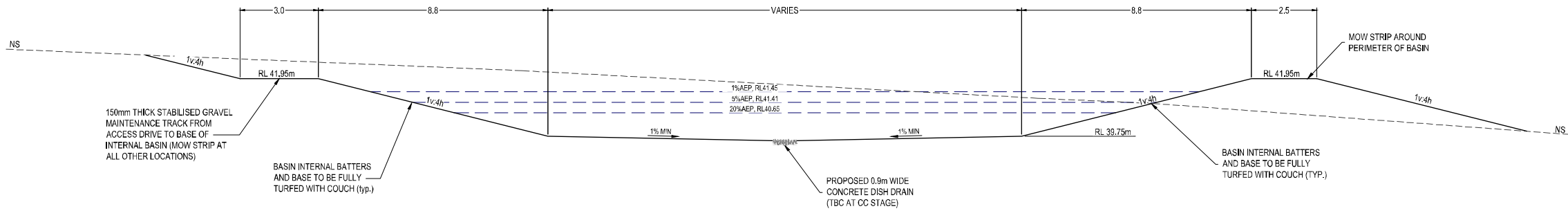


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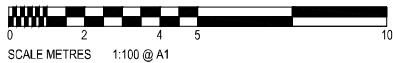




SECTION - A  
1:100



SECTION - B  
1:100



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B	COUNCIL AMENDMENTS - PREL S34	19.08.24	K.B.	
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PO Box 1138  
HUNTERS HILL NSW 2110



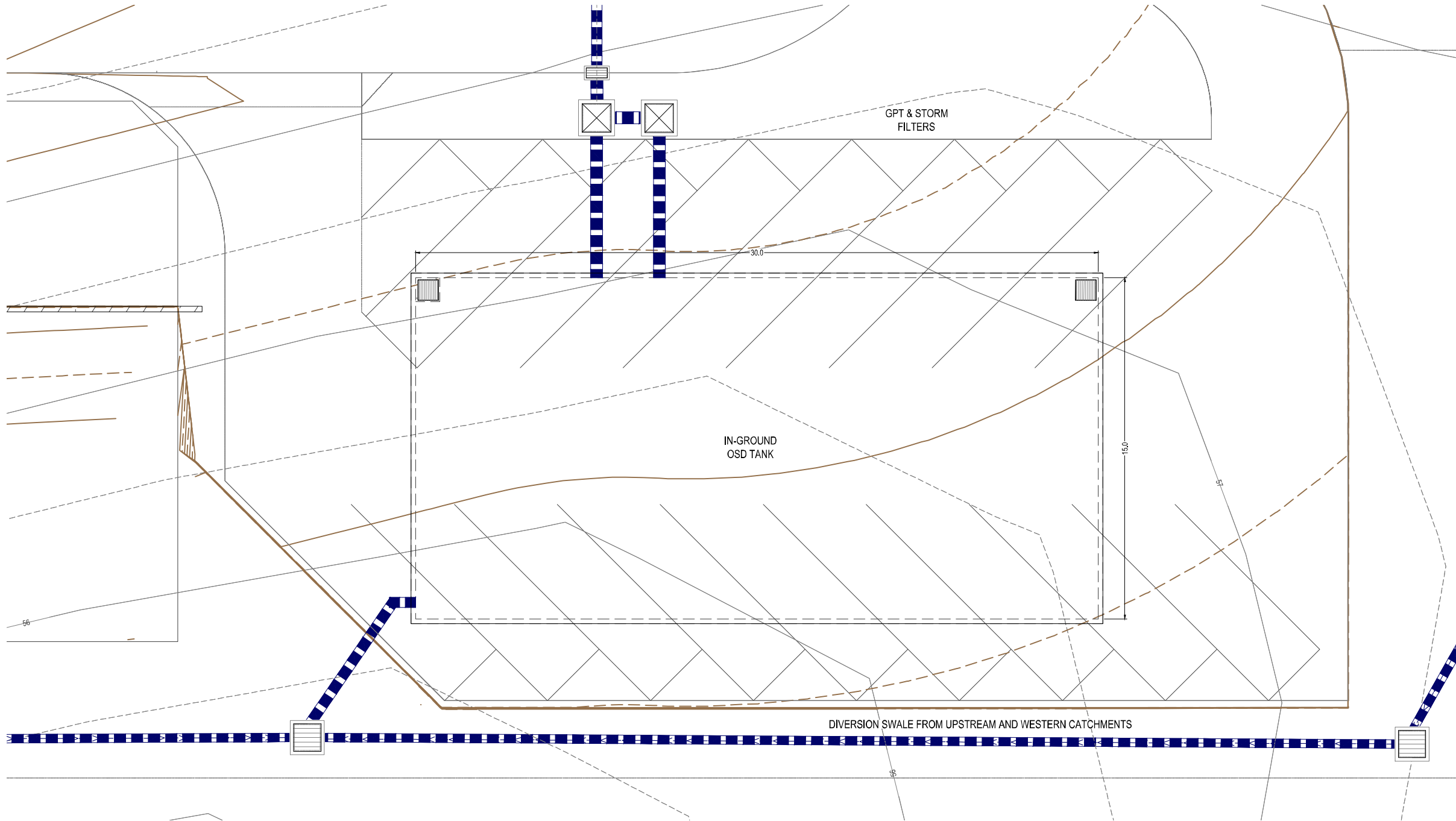
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PROJECT:  
CIVIL WORKS for  
LOT 225, DP 246447  
34 WYNDELLA ROAD  
LOCHINVAR NSW 2321

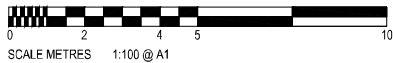
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**CIVIL SERVICES**  
ON-SITE DETENTION BASIN No.1  
TYPICAL SECTIONS

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PROJECT No. 23128			DRAWING No. C06.03			





ON-SITE DETENTION TANK No. 2 PLAN  
1:100



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REV	DESCRIPTION	DATE	APP'D	END'D
D	LEC AMENDMENTS	01.05.25	K.B.	
C	COUNCIL AMENDMENTS - S34 OUTCOMES	25.11.24	K.B.	
B	COUNCIL AMENDMENTS - PREL S34	19.08.24	K.B.	
A	ISSUED FOR DA APPROVAL	02.02.24	K.B.	

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PROJECT:  
**CIVIL WORKS** for  
LOT 225, DP 246447  
34 WYNDELLA ROAD  
LOCHINVAR NSW 2321

DRAWING TITLE:  
**CIVIL SERVICES**  
ON-SITE DETENTION TANK No. 2 PLAN

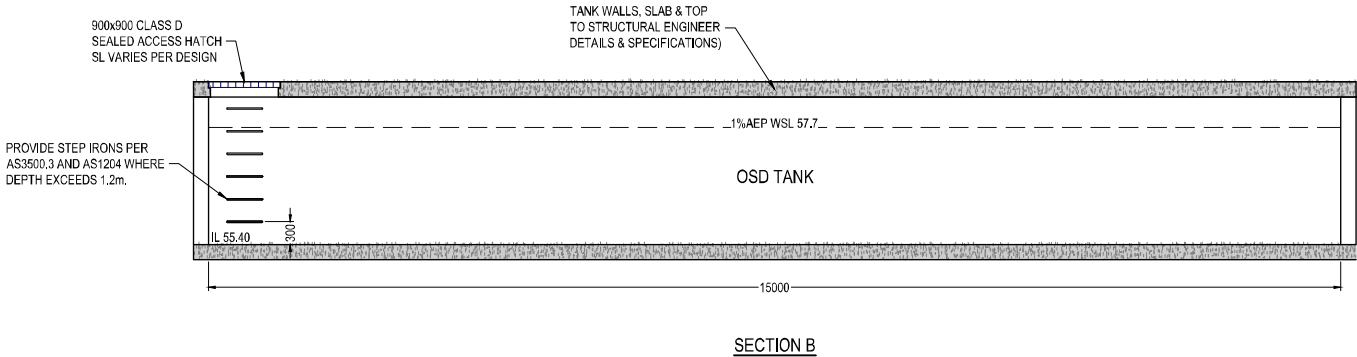
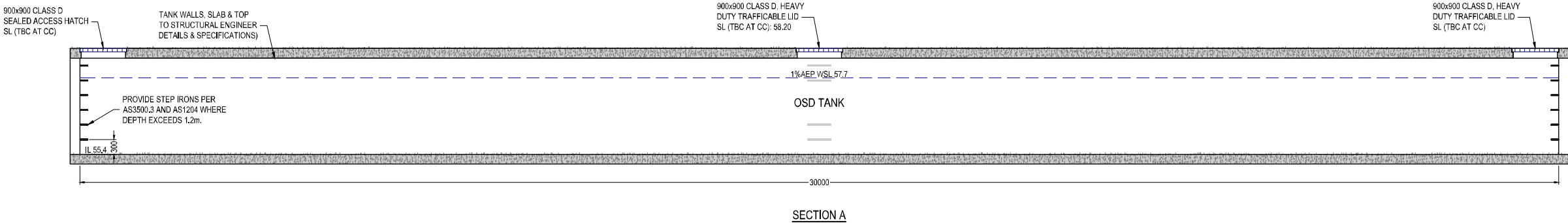
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SCALE: 1:100					ORIG. SIZE: <b>A1</b>
DRAWN: MS	DESIGNED: KB	CHECKED: AS	APPROVED: KB	ENDORSED: -	DATE: 01.05.25
PROJECT No.: <b>23128</b>		DRAWING No.: <b>C06.04</b>			REV: <b>D</b>

PLOTTED BY: KEN    DATE: 1 May 2025 4:29 PM    FILENAME: C:\PROJECTS\2023\23128 - 34 WYNDELLA ROAD, LOCHINVAR\CAD\TTL\23128\_CIV\_DL\EL\_34 WYNDELLA ROAD



BASIN AND OUTLET CONTROL STRUCTURE NOTES:

TOTAL BASIN STORAGE	1035 m <sup>3</sup>
WATER SURFACE LEVELS	
20% AEP	56.57 AHD
5% AEP	57.05 AHD
1% AEP	57.66 AHD
OUT CONTROL STRUCTURE	
ORIFICE 1	Ø250 AT 55.4 AHD
ORIFICE 2	Ø250 AT 56.6 AHD
EMERGENCY OVERFLOW WEIR	1.8m AT 57.7 AHD
OUTLET PIPE FROM TANK	450mm RCP



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LOCHINVAR NSW 2321

DRAWING TITLE:  
**CIVIL SERVICES**  
ON-SITE DETENTION TANK No. 2  
TYPICAL SECTIONS

DRAWING STATUS <b>DA APPROVAL</b> NOT TO BE USED FOR CONSTRUCTION						
SCALE: 1:50					ORIG. SIZE A1	
DRAWN MS	DESIGNED KB	CHECKED AS	APPROVED KB	ENDORSED -	DATE	REV
PROJECT No. 23128					DRAWING No. C06.05	D





# ROAD IMPROVEMENT for WYNDELLA ROAD NEW ENGLAND HWY TO LOT 225, DP246447 LOCHIVAR NSW 2321

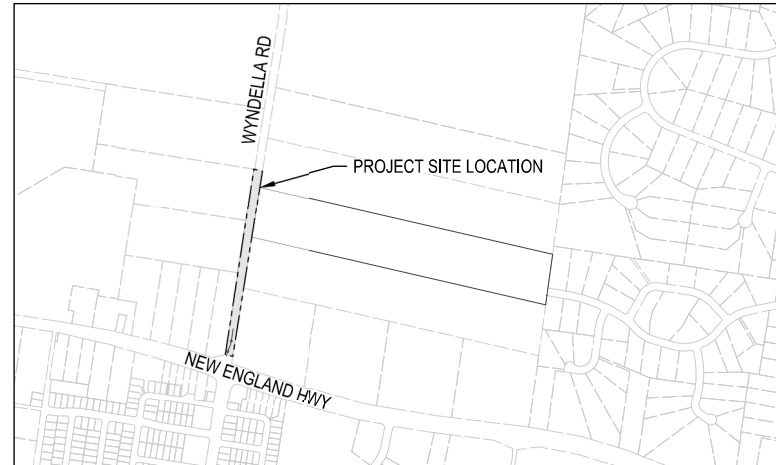
Prepared by  
**WALLACE INFRASTRUCTURE DESIGN PTY LTD**

KENNETH D BOYER  
(BE Civil) MIE Aust CPEng

Signature:

Chartered Professional Engineer  
Membership No. 4404352

Registered on the NER in the area(s)  
of practice of CIVIL.



LOCATION PLAN  
(NTS)

DRAWING NUMBER	DRAWING DESCRIPTION
C01.01	LEGEND, DRAWING SCHEDULE AND LOCATION PLAN
C02.01	GENERAL NOTES
C02.02	GENERAL ARRANGEMENT PLAN
C03.01	EROSION AND SEDIMENTATION CONTROL PLAN
C03.02	EROSION AND SEDIMENTATION CONTROL DETAILS
C04.01	WYNDELLA ROAD DETAIL PLAN AND LONGITUDINAL SECTION
C04.02	WYNDELLA ROAD DETAIL PLAN AND LONGITUDINAL SECTION
C04.03	WYNDELLA ROAD CROSS-SECTIONS
C04.04	WYNDELLA ROAD CROSS-SECTIONS
C04.05	WYNDELLA ROAD CROSS-SECTIONS
C04.06	WYNDELLA ROAD CROSS-SECTIONS
C04.07	WYNDELLA ROAD CROSS-SECTIONS
C04.08	WYNDELLA ROAD CROSS-SECTIONS
C04.09	LOT 223 & LOT 224 DRIVEWAY AND BUSHFIRE ACCESS LONGITUDINAL SECTIONS
C05.01	STORMWATER CATCHMENT PLAN
C05.02	STORMWATER MANAGEMENT PLAN
C05.03	STORMWATER LONGITUDINAL SECTIONS
C06.01	TYPICAL ROAD SECTIONS
C06.02	TYPICAL CULVERT SECTIONS AND DETAILS

## ABBREVIATIONS

A/G	ABOVE GROUND	IFC	ISSUED FOR CONSTRUCTION	SV	STOP VALVE
AGG	AGGREGATE	IFCR	ISSUED FOR CLIENT REVIEW	SW	STORMWATER
ARCH	ARCHITECT	IL	INVERT LEVEL	SWP	STORMWATER PIT
ASP	ASPHALT	IO	INSPECTION OPENING	THK	THICKNESS
BM	BENCHMARK	KIP	KERB INLET PIT	TJ	TRAVERSE JOINT
CAD	COMPUTER AIDED DRAFTING	L	LENGTH	TOW	TOP OF WALL
C-C	CENTRE TO CENTRE	LP	LAMP POST (SURVEY)	TW	TRADE WASTE
CH	CHAINAGE	MAX	MAXIMUM	TYP	TYPICAL
CI	CAST IRON	MC	MASS CONCRETE	U/G	UNDERGROUND
CJ	CONSTRUCTION JOINT	MH	MANHOLE	UNO	UNLESS NOTED OTHERWISE
CL	COVER LEVEL or CENTRE LINE	MIN	MINIMUM	U/S	UPSTREAM
CO	CLEAN OUT (SUBSOIL)	MISC	MISCELLANEOUS	VC	VITRIFIED CLAY PIPE
COG	CHANGE OF GRADE	N	NORTH	VP	VENT PIPE
CONC	CONCRETE	NRV	NON RETURN VALVE	VR	VERTICAL RISER
CTRL	CONTROL	NTS	NOT TO SCALE	W	WIDTH
CR	CROWN	OD	OUTER DIAMETER	WC	WATER CLOSET
CTS	CENTRES	OF	OVERFLOW	WL	WATER LEVEL
D	DEPTH	OH	OVERHEAD	WM	WATER METER
DIA	DIAMETER	PP	POWER POLE	WS	WASTE STACK
DP	DOWNPIPE	PPE	PERSONAL PROTECTIVE EQUIP.		
DRG	DRAWING	PVC	POLYVINYLCHLORIDE	UNITS	
D/S	DOWN STREAM	PV	PRESSURE VENT	mm	MILLIMETRES
DTM	DIGITAL TERRAIN MODEL	PVP	PRESSURE VENT PIPE	cm	CENTIMETRES
EB	EDGE BITUMIN	QA	QUALITY ANALYSIS	m	METRES
EX	EXISTING	QTY	QUANTITY	m <sup>2</sup>	SQUARE METRES
ESL	EXISTING SURFACE LEVEL	R	RADIUS	m <sup>3</sup>	CUBIC METRES
FH	FIRE HYDRANT	RC	REINFORCED CONCRETE	L/s	LITRES PER SECOND
FHR	FIRE HOSE REEL	REV	REVISION	ha	HECTARES
FFL	FINISHED FLOOR LEVEL	RL	REDUCED LEVEL		
FGL	FINISHED GROUND LEVEL	RW	RETAINING WALL		
FSL	FINISHED SURFACE LEVEL	SFW	SEALED FLOOR WASTE		
FW	FLOOR WASTE	SL	SURFACE LEVEL		
GA	GENERAL ARRANGEMENT	SMH	SEWER MANHOLE		
GI	GALVANISED IRON	SMV	SEWER MAIN VENT		
GFL	GARAGE FINISHED LEVEL	SOD	SIDE OUTLET DRAIN		
IC	INSPECTION CHAMBER	S/S	STAINLESS STEEL		
ID	INSIDE DIAMETER	STD	STANDARD		

## LINETYPES & SYMBOLS

### GENERAL

SITE BOUNDARY	---
CADASTRAL BOUNDARY	---
EASEMENT BOUNDARY	---
EXISTING FEATURES	
CONTOURS	---30---
STORMWATER PIPE	---
STORMWATER PIT	■
ELECTRICAL CABLE - U/G	---E---
ELECTRICAL CABLE - O/H	---OHE---
EXISTING SEWER PIPE	---S---
EXISTING WATER	---W---
EXISTING WATER (HYDRANT)	●
EXISTING WATER (STOP VALVE)	X
EXISTING TELECOM	---OFC---
COMMUNICATIONS PIT	■
EXISTING GAS	---G---
EXISTING BUILDING	---
TREES / SHRUBS	○ ○ ○ ○
PROPOSED - BUILDING (INDICATIVE ONLY)	
DWELLING NUMBER	UNIT 01
FINISHED FLOOR LEVEL	FFL 5.00
BUILDING FOOTPRINT	---

### PROPOSED - UTILITIES

SEWER	---S---
WATER	---W---
COMMUNICATIONS	---T---
COMMUNICATIONS - OPTIC FIBRE	---OFC---
COMMUNICATIONS PIT	■
GAS	---G---
ELECTRICAL CABLE - U/G	---E---
ELECTRICAL CABLE - O/H	---OHE---
REDUNDANT	· X · X · X · X · X ·
PROPOSED - EROSION AND SEDIMENT CONTROL	
SITE EXCLUSION FENCE	— [X] — [X] — [X] —
SEDIMENT FENCE	— [ ] — [ ] — [ ] —
DIVERSION DRAIN	— > > > —
STABILISED SITE ACCESS	■ ■ ■ ■
MATERIAL STOCKPILE	■ ■ ■ ■
SLOPE DIRECTION	→
GEOTEXTILE INLET FILTER	■ ■ ■ ■
MESH AND GRAVEL INLET FILTER	■ ■ ■ ■
SANDBAG / HAY BALES	■ ■ ■ ■
PROPOSED CONTOURS	
MAJOR CONTOUR INTERVAL	---10---
MINOR CONTOUR INTERVAL	---

### PROPOSED - STORMWATER

SW PIPE (INCL. DIA/GRADE/LENGTH)	ØXXXmm uPVC X.X% GRADE, X.XXm
SW PIPE - CHARGED (INCL. DIA/GRADE/LENGTH)	ØXXXmm uPVC CHARGE X.X% GRADE, X.XXm
SW PIT - GRATED / JUNCTION	■ ■
SW - KERB INLET PIT (INCLUDING INTEL)	■
RAINWATER/REUSE TANK	○ OR ■
GROSS POLLUTANT TRAP	○ GPT
GRATED DRAIN	---
INFILTRATION TRENCH	---
HEADWALL	■
SUBSOIL	---
SW SWALE	---
PIPE RISER	○ XX
PIPE DROPPER	○ XX
OVERLAND FLOW PATH	→
SW PIT NUMBER	?
SW CATCHMENT (NUMBER / AREA IN ha)	XX XXXX

### PROPOSED - CIVIL

CONCRETE	■
ASPHALTIC CONCRETE	■
SAND	■
EARTH	■
RIP RAP	■
BUILDING / STRUCTURES	■
TIMBER	■
BLOCK PAVERS	■
RETAINING WALL	■
KERB RAMP	■
VEHICULAR CROSSING	■
FENCE	---
BATTER	---
BOLLARD - TYPE 1 (FIXED)	● B-T1
BOLLARD - TYPE 2 (REMOVABLE)	● B-T2
LINEMARKING - CHEVRON	■ ■ ■ ■
EXPANSION JOINT	---EJ---
TRAVERSE JOINT	---TJ---
CONTROL JOINT	---CJ---
FINISHED GRADE LEVELS	25.758
EXISTING GRADE LEVELS	25.710

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A	PRELIMINARY ISSUE	01.05.25	K.B.	

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PROJECT:

ROAD IMPROVEMENTS for WYNDELLA ROAD  
LOT 225, DP 246447  
34 WYNDELLA ROAD  
LOCHIVAR NSW 2321

DRAWING TITLE:

CIVIL ROAD & SW PLANS  
LEGEND, DRAWING SCHEDULE  
AND LOCATION PLAN

DRAWING STATUS

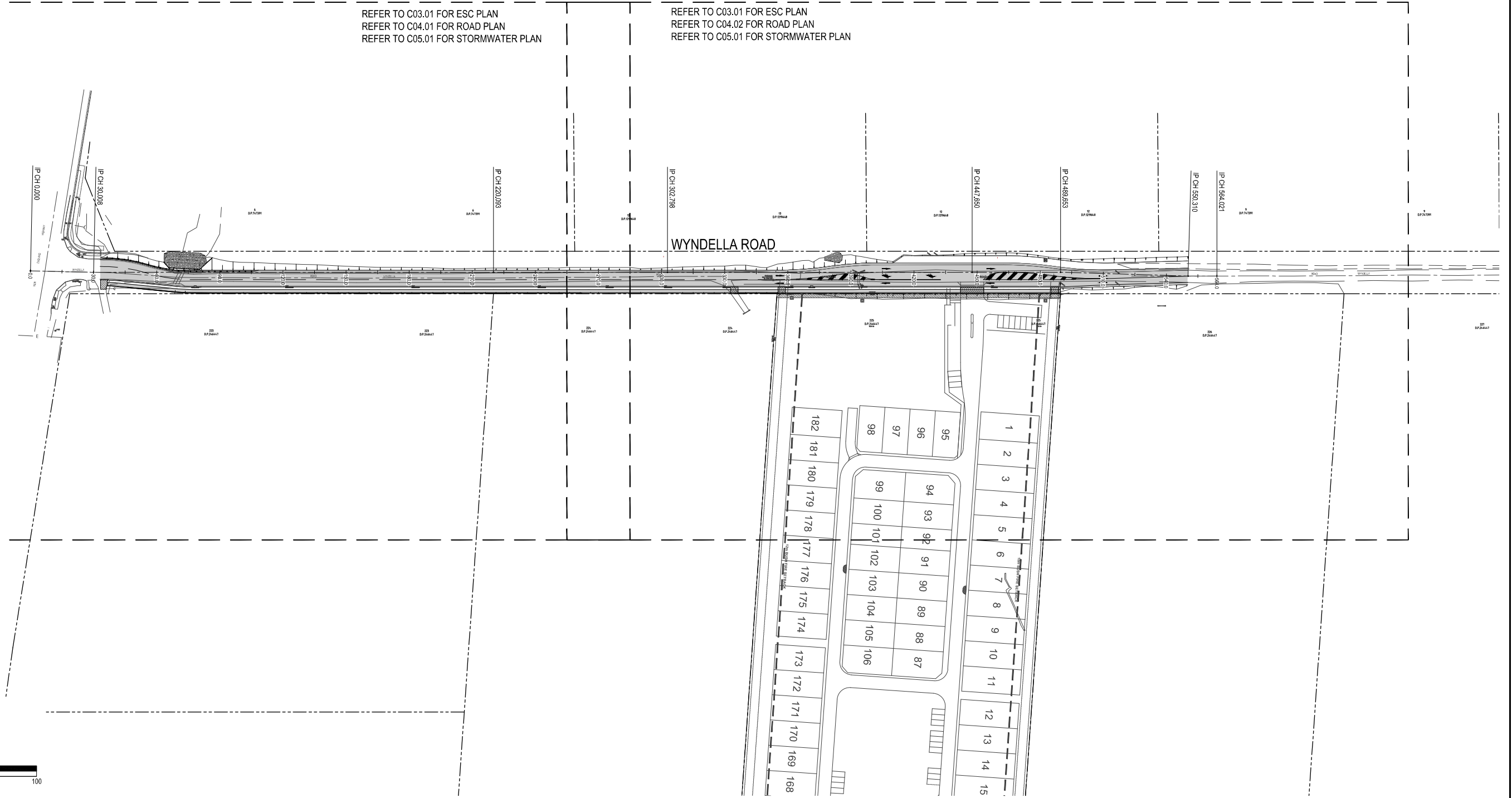
PRELIMINARY DESIGN

SCALE:	N.T.S.	ORIG. SIZE	A1
DRAWN	DESIGNED	CHECKED	APPROVED
MS	KB	KB	KB
PROJECT No:			DRAWING No:
23128			C01.01
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SCALE METRES 1:1000 @ A1

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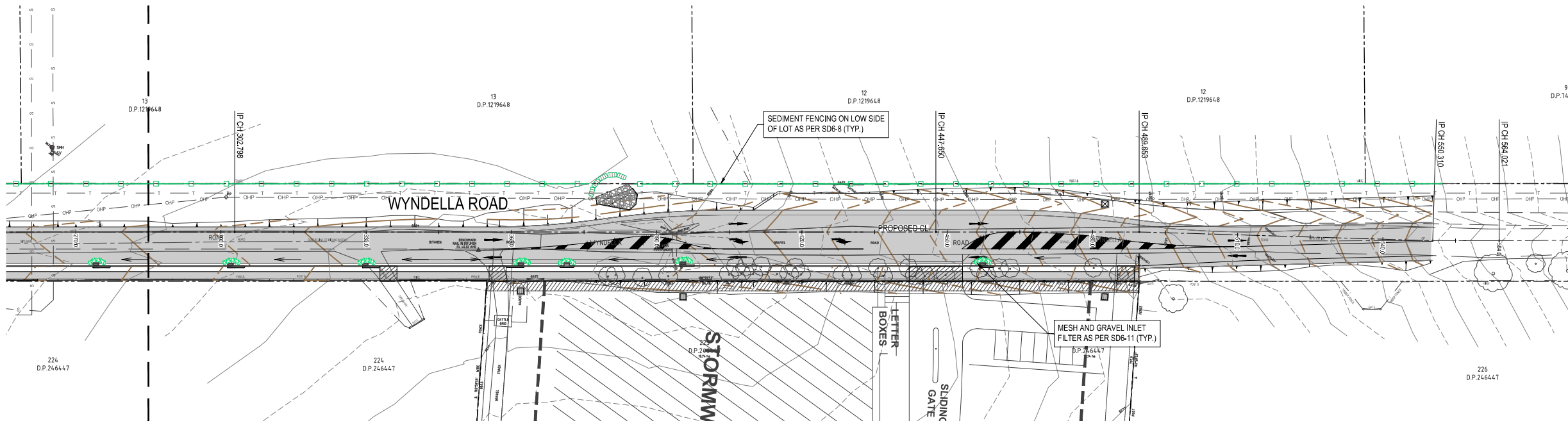
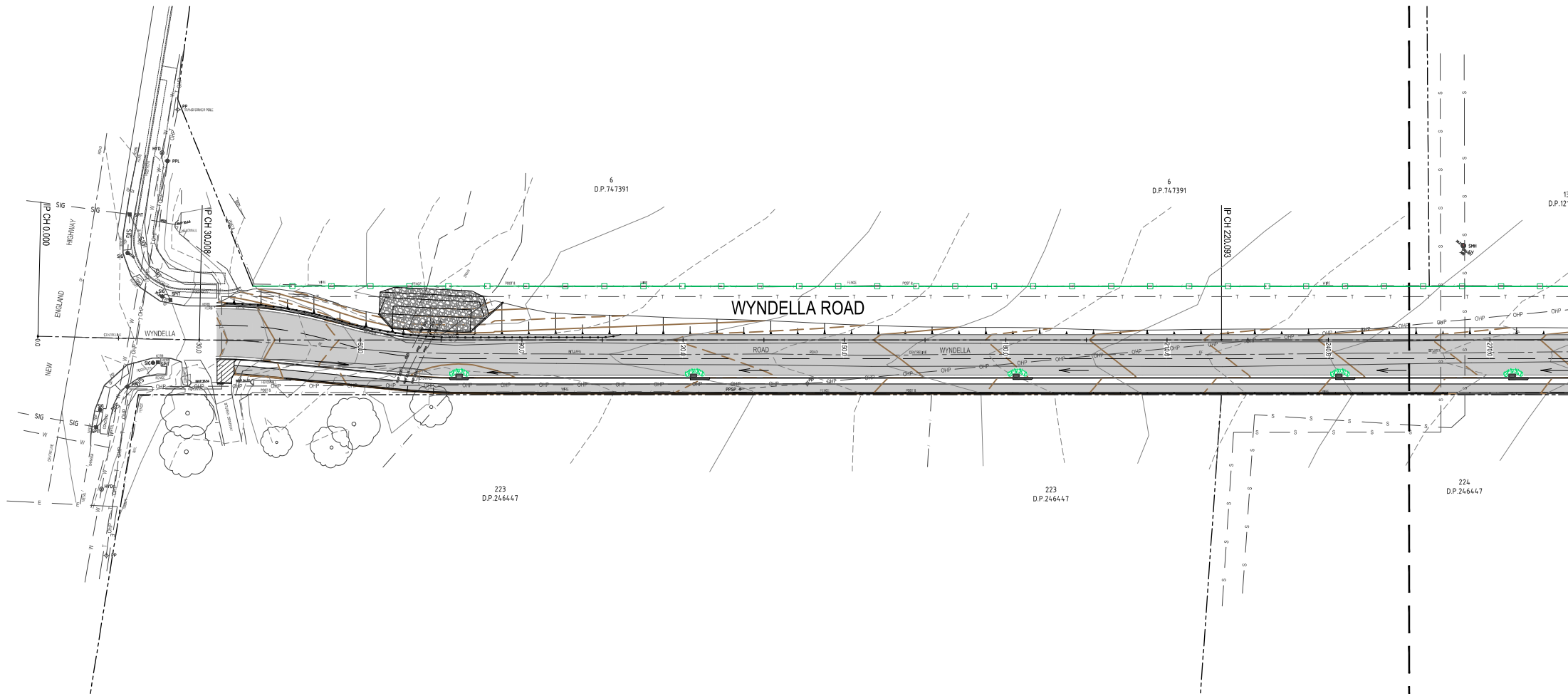
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GENERAL ARRANGEMENT PLAN

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MS	KB	KB	KB	-	05.05.25	
PROJECT No:				DRAWING No:		REV
23128				C03.01		B



NOTE

1. ALL EROSION & SEDIMENTATION CONTROL (ESC) PLAN DETAILS SHOWN ARE FROM "MANAGING URBAN STORMWATER - SOILS AND CONSTRUCTION", LANDCOM, 2004. DETAILS, ALTHOUGH NOT SPECIFICALLY IDENTIFIED FOR USE ON THE ESC PLAN (SHEET C03.01), ARE PROVIDED IN THE EVENT ADDITIONAL CONTROLS ARE REQUIRED TO MINIMISE ON-SITE EROSION DURING THE COURSE OF CONSTRUCTION. IF AN ESC DETAIL IS NOT SHOWN, CONTRACTOR SHALL REFER TO THE ABOVE REFERENCED GUIDE.

**CONSTRUCTION NOTES:**

- PLACE STOCKPILES MORE THAN 2 (PREFERABLY 5) METERS FROM EXISTING VEGETATION, CONCENTRATED WATER FLOW, ROADS AND HAZARD AREAS.
- CONSTRUCT ON THE CONTOUR AS LOW, FLAT, ELONGATED MOUNDS.
- WHERE THERE IS SUFFICIENT AREA, TOPSOIL STOCKPILES SHALL BE LESS THAN 2 METERS IN HEIGHT.
- WHERE THEY ARE TO BE IN PLACE FOR MORE THAN 10 DAYS, STABILISE FOLLOWING THE APPROVED ESCP OR SWMP TO REDUCE THE C-FACTOR TO LESS THAN 0.10.
- CONSTRUCT EARTH BANKS (STANDARD DRAWING 5-5) ON THE UPSLOPE SIDE TO DIVERT WATER AROUND STOCKPILES AND SEDIMENT FENCES (STANDARD DRAWING 6-4) 1 TO 2 METERS DOWNSLOPE.

STOCKPILES

SD 4-1

**CONSTRUCTION NOTES:**

- SCARIFY THE GROUND SURFACE ALONG THE LINE OF THE CONTOUR TO A DEPTH OF 50mm TO 100mm TO BREAK UP ANY HARDSETTING SURFACES AND TO PROVIDE A GOOD BOND BETWEEN THE RESPREAD MATERIAL AND SUBSOIL.
- ADD SOIL AMELIORANTS AS REQUIRED BY THE ESCP OR SWMP.
- RIP TO A DEPTH OF 300mm IF COMPACTED LAYERS OCCUR.
- WHERE POSSIBLE: REPLACE TOPSOIL TO A DEPTH OF 40 TO 60mm ON LANDS WHERE THE SLOPE EXCEEDS 4(H):1(V) AND TO AT LEAST 75mm ON LOWER GRADIENTS.

REPLACING TOPSOIL

SD 4-2

**CONSTRUCTION NOTES:**

- CHECK DAMS CAN BE BUILT WITH VARIOUS MATERIALS, INCLUDING ROCKS, LOGS, SANBAGS AND STRAW BALES. THE MAINTENANCE PROGRAM SHOULD ENSURE THEIR INTEGRITY IS RETAINED, ESPECIALLY WHERE CONSTRUCTED WITH STRAW BALES. IN THE CASE OF BALES, THIS MIGHT REQUIRE THEIR REPLACEMENT EACH TWO TO FOUR MONTHS.
- TRENCH THE CHECK DAM 200mm INTO THE GROUND ACROSS ITS WHOLE WIDTH. WHERE ROCK IS USED, FILL THE TRENCHES TO AT LEAST 100mm ABOVE THE GROUND SURFACE TO REDUCE THE RISK OF UNDERCUTTING.
- NORMALLY, THEIR MAXIMUM HEIGHT SHOULD NOT EXCEED 600mm ABOVE THE GULLY FLOOR. THE CENTRE SHOULD ACT AS A SPILLWAY, BEING AT LEAST 150mm LOWER THAN THE OUTER EDGE.
- SPACE THE DAMS SO THE TOE OF THE UPSTREAM DAM IS LEVEL WITH THE SPILLWAY OF THE NEXT DOWNSTREAM DAM.

ROCK CHECK DAM

SD 5-4

**CONSTRUCTION NOTES:**

- BUILD WITH GRADIENTS BETWEEN 1 PERCENT AND 5 PERCENT.
- AVOID REMOVING TREES AND SHRUBS IF POSSIBLE - WORK AROUND THEM.
- ENSURE THE STRUCTURES ARE FREE OF PROJECTIONS OR OTHER IRREGULARITIES THAT COULD IMPEDE WATER FLOW.
- BUILD THE DRAINS WITH CIRCULAR, PARABOLIC OR TRAPEZOIDAL CROSS SECTIONS, NOT V-SHAPED.
- ENSURE THE BANKS ARE PROPERLY COMPACTED TO PREVENT FAILURE.
- COMPLETE PERMANENT OR TEMPORARY STABILISATION WITHIN 10 DAYS OF CONSTRUCTION.

EARTH BANK (LOW FLOW)

SD 5-5

**CONSTRUCTION NOTES:**

- CONSTRUCT THE STRAW BALE FILTER AS CLOSE AS POSSIBLE TO BEING PARALLEL TO THE CONTOURS OF THE SITE.
- PLACE BALES LENGTHWISE IN A ROW WITH ENDS TIGHTLY ABUTTING. USE STRAW TO FILL ANY GAPS BETWEEN BALES. STRAWS ARE TO BE PLACED PARALLEL TO GROUND.
- ENSURE THAT THE MAXIMUM HEIGHT OF THE FILTER IS ONE BALE.
- EMBED EACH BALE IN THE GROUND 75mm TO 100mm AND ANCHOR WITH TWO 1.2m STAR PICKETS OR STAKES. ANGLE THE FIRST STAR PICKET OR STAKE IN EACH BALE TOWARDS THE PREVIOUSLY LAID BALE. DRIVE THEM 600mm INTO THE GROUND AND, IF POSSIBLE, FLUSH WITH THE TOP OF THE BALES. WHERE STAR PICKETS ARE USED AND THEY PROTRUDE ABOVE THE BALES, ENSURE THEY ARE FITTED WITH SAFETY CAPS.
- WHERE A STRAW BALE FILTER IS CONSTRUCTED DOWNSLOPE FROM A DISTURBED BATTER, ENSURE THE BALES ARE PLACED 1 TO 2 METRES DOWNSLOPE FROM THE TOE.
- ESTABLISH A MAINTENANCE PROGRAM THAT ENSURES THE INTEGRITY OF THE BALES IS RETAINED - THEY COULD REQUIRE REPLACEMENT EACH TWO TO FOUR MONTHS.

STRAW BALE FILTER

SD 6-7

**CONSTRUCTION NOTES:**

- CONSTRUCT SEDIMENT FENCES AS CLOSE AS POSSIBLE TO BEING PARALLEL TO THE CONTOURS OF THE SITE, BUT WITH SMALL RETURNS AS SHOWN IN THE DRAWING TO LIMIT THE CATCHMENT AREA OF ANY ONE SECTION. THE CATCHMENT AREA SHOULD BE SMALL ENOUGH TO LIMIT WATER FLOW IF CONCENTRATED AT ONE POINT TO 50 LITRES PER SECOND IN THE DESIGN STORM EVENT, USUALLY THE 10-YEAR EVENT.
- CUT A 150mm DEEP TRENCH ALONG THE UPSLOPE LINE OF THE FENCE FOR THE BOTTOM OF THE FABRIC TO BE ENTRENCHED.
- DRIVE 1.5 METRE LONG STAR PICKETS INTO GROUND AT 2.5 METRE INTERVALS (MAX) AT THE DOWNSLOPE EDGE OF THE TRENCH. ENSURE ANY STAR PICKETS ARE FITTED WITH SAFETY CAPS.
- FIX SELF-SUPPORTING GEOTEXTILE TO THE UPSLOPE SIDE OF THE POSTS ENSURING IT GOES TO THE BASE OF THE TRENCH. FIX THE GEOTEXTILE WITH WIRE TIES OR AS RECOMMENDED BY THE MANUFACTURER. ONLY USE GEOTEXTILE SPECIFICALLY PRODUCED FOR SEDIMENT FENCING. THE USE OF SHADE CLOTH FOR THIS PURPOSE IS NOT SATISFACTORY.
- JOIN SECTIONS OF FABRIC AT A SUPPORT POST WITH A 150mm OVERLAP.
- BACKFILL THE TRENCH OVER THE BASE OF THE FABRIC AND COMPACT IT THOROUGHLY OVER THE GEOTEXTILE.

SEDIMENT FENCE

SD 6-8

**CONSTRUCTION NOTES:**

- INSTALL FILTERS TO KERB INLETS ONLY AT SAG POINTS.
- FABRICATE A SLEEVE MADE FROM GEOTEXTILE OR WIRE MESH LONGER THAN THE LENGTH OF THE INLET PIT AND FILL IT WITH 25mm TO 50mm GRAVEL.
- FORM AN ELLIPTICAL CROSS-SECTION ABOUT 150mm HIGH X 400mm WIDE.
- PLACE THE FILTER AT THE OPENING LEAVING AT LEAST A 100mm SPACE BETWEEN IT AND THE KERB INLET. MAINTAIN THE OPENING WITH SPACER BLOCKS.
- FORM A SEAL WITH THE KERB TO PREVENT SEDIMENT BYPASSING THE FILTER.
- SANBAGS FILLED WITH GRAVEL CAN SUBSTITUTE FOR THE MESH OR GEOTEXTILE PROVIDING THEY ARE PLACED SO THAT THEY FIRMLY ABUT EACH OTHER AND SEDIMENT-LADEN WATERS CANNOT PASS BETWEEN.

MESH AND GRAVEL INLET FILTER

SD 6-11

**CONSTRUCTION NOTES:**

- FABRICATE A SEDIMENT BARRIER MADE FROM GEOTEXTILE OR STRAW BALES.
- FOLLOW STANDARD DRAWING 6-7 AND STANDARD DRAWING 6-8 FOR INSTALLATION PROCEDURES FOR THE STRAW BALES OR GEOTEXTILE. REDUCE THE PICKET SPACING TO 1 METRE CENTRES.
- IN WATERWAYS, ARTIFICIAL SAG POINTS CAN BE CREATED WITH SANBAGS OR EARTH BANKS AS SHOWN IN THE DRAWINGS.
- DO NOT COVER THE INLET WITH GEOTEXTILE UNLESS THE DESIGN IS ADEQUATE TO ALLOW FOR ALL WATERS TO BYPASS IT.

GEOTEXTILE INLET FILTER

SD 6-12

**CONSTRUCTION NOTES:**

- INSTALL A 400mm MINIMUM WIDE ROLL OF TURF ON THE FOOTPATH NEXT TO THE KERB AND AT THE SAME LEVEL AS THE TOP OF THE KERB AND LIGHTLY COMPACTED INTO THE TOPSOIL.
- LAY 1.4 METRE LONG TURF STRIPS NORMAL TO THE KERB EVERY 10 METRES.
- REHABILITATE DISTURBED SOIL BEHIND THE KERB BY SPREADING 20-25mm THICK LAYER OF TOPSOIL AND BROADCAST SEED MIX AT A RATE OF 0.3kg/10m2. UNLESS SPECIFIED OTHERWISE, TO DISTURBED AREAS WHERE TURF HAS NOT BEEN INSTALLED. SEEDS SHOULD BE SCRATCHED INTO THE SURFACE TO REDUCE THE INCIDENCE OF INGESTION BY BIRDS. IF SEED MIX DOES NOT INCLUDE STARTER FERTILISER, APPLY FERTILISER AT A RATE AS SPECIFIED.

KERBSIDE TURF STRIP

SD 6-13

**CONSTRUCTION NOTES:**

- STRIP THE TOPSOIL, LEVEL THE SITE AND COMPACT THE SUBGRADE.
- COVER THE AREA WITH NEEDLE-PUNCHED GEOTEXTILE.
- CONSTRUCT A 200mm THICK PAD OVER THE GEOTEXTILE USING ROAD BASE OR 30mm AGGREGATE.
- ENSURE THE STRUCTURE IS AT LEAST 15 METRES LONG OR TO BUILDING ALIGNMENT AND AT LEAST 3 METRES WIDE.
- WHERE A SEDIMENT FENCE JOINS ONTO THE STABILISED ACCESS, CONSTRUCT A HUMP IN THE STABILISED ACCESS TO DIVERT WATER TO THE SEDIMENT FENCE.

STABILISED SITE ACCESS

SD 6-14

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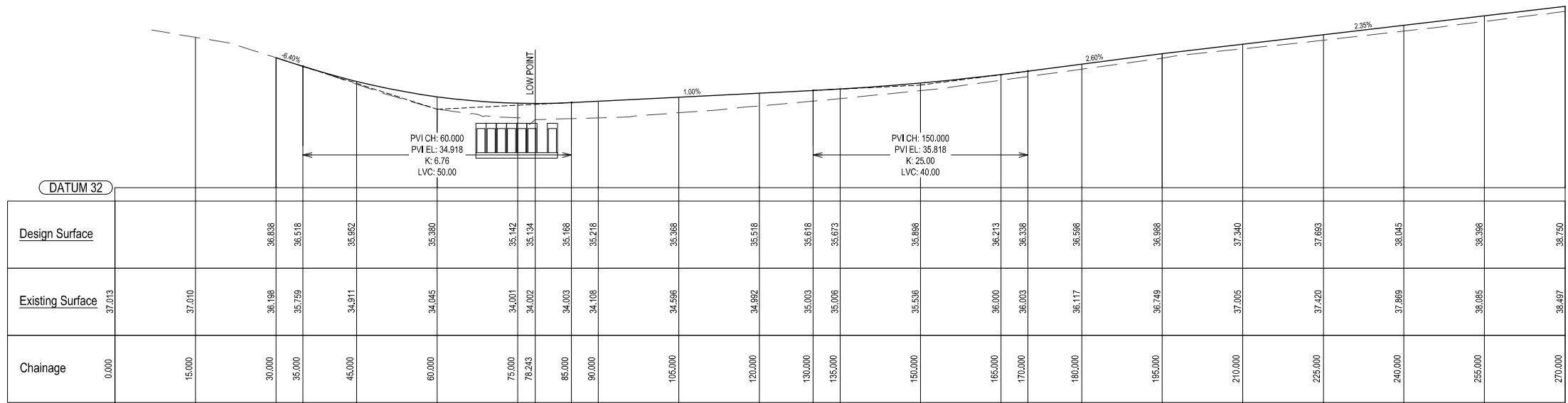
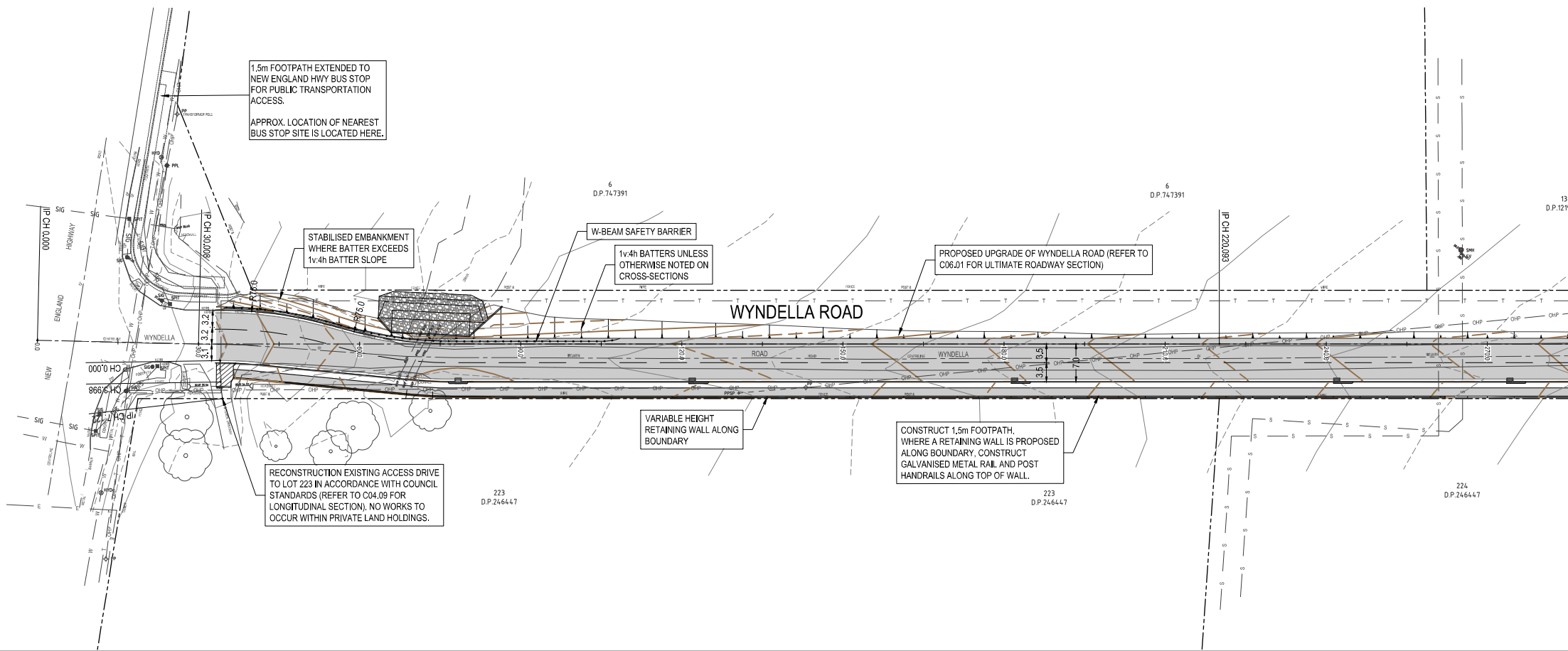
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EROSION AND SEDIMENTATION  
CONTROL DETAILS

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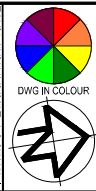


LONGITUDINAL SECTION  
Wyndella Road Upgrade



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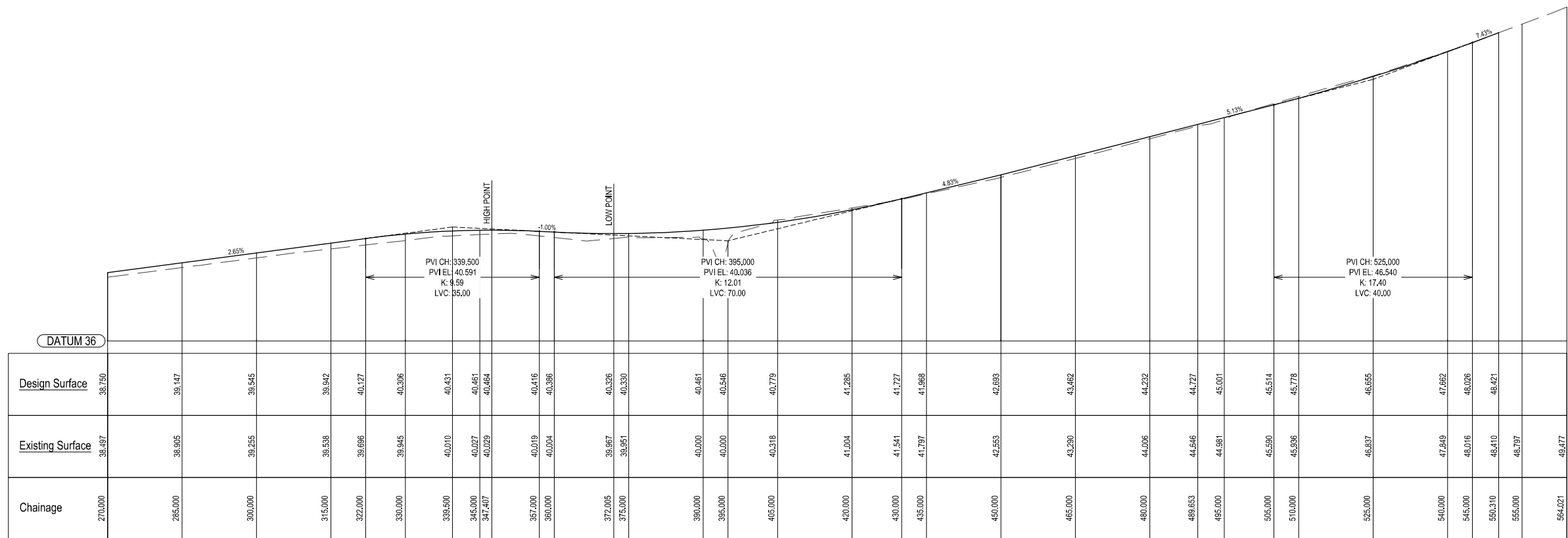
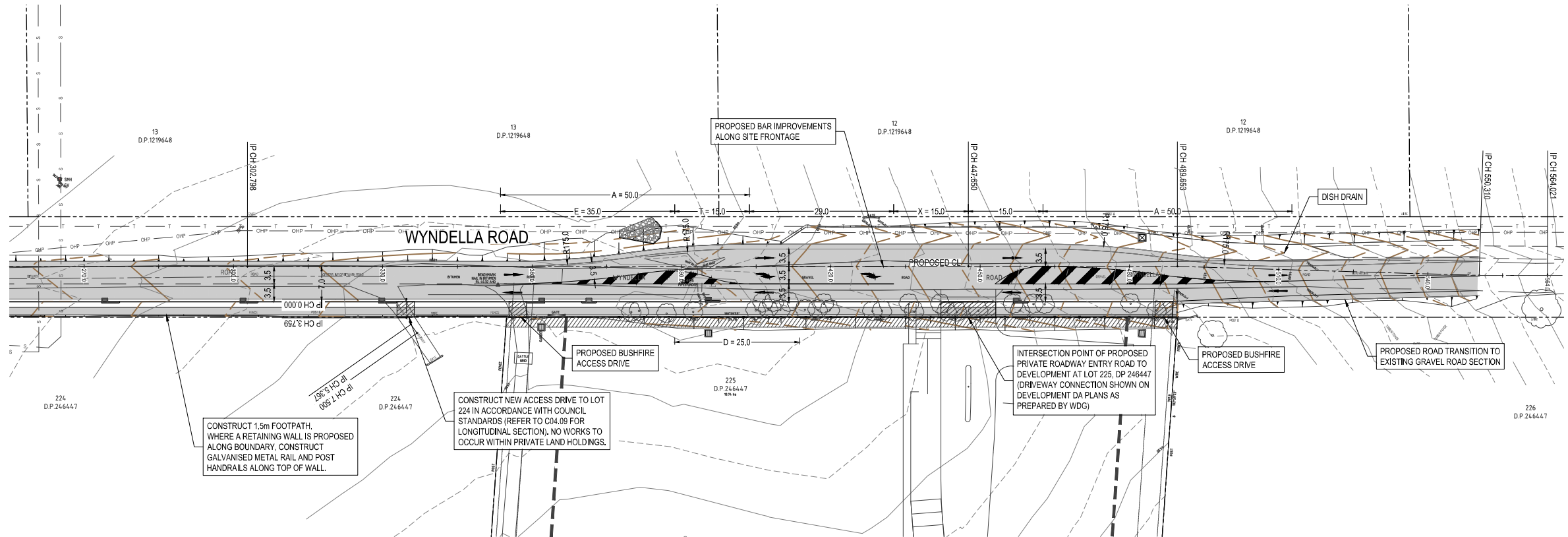
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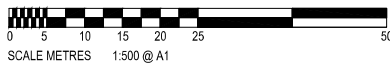
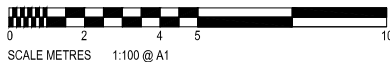
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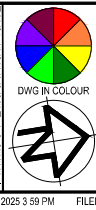


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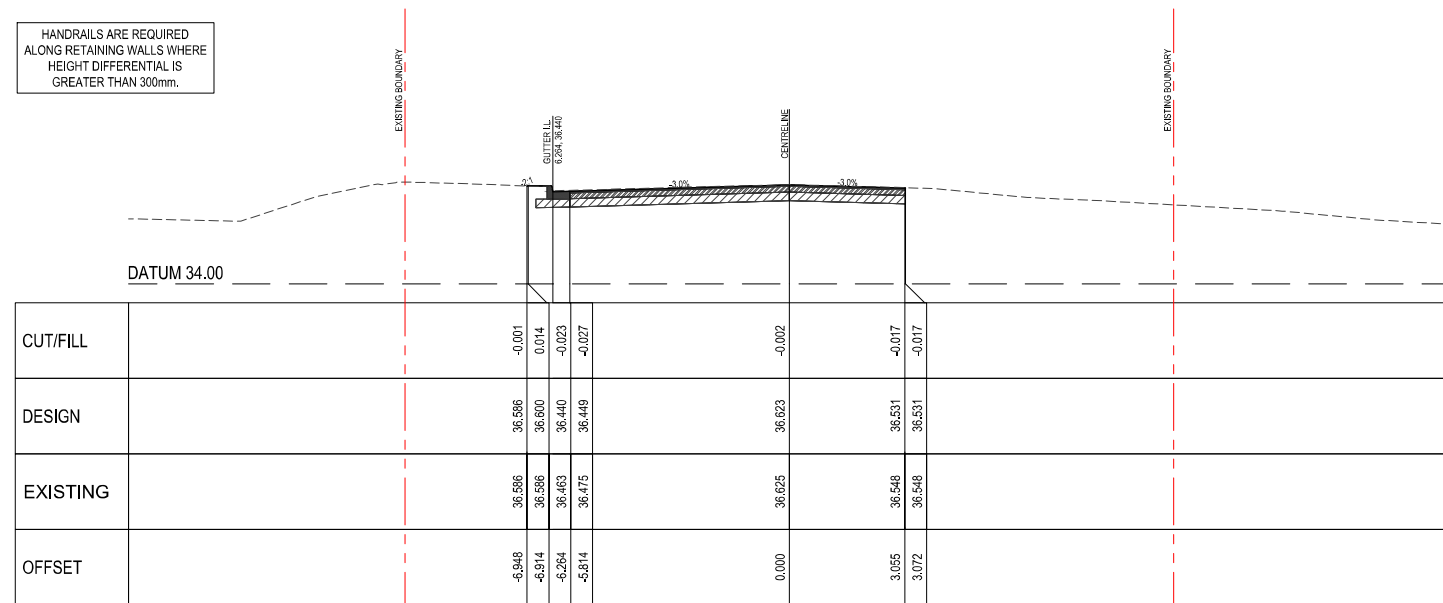
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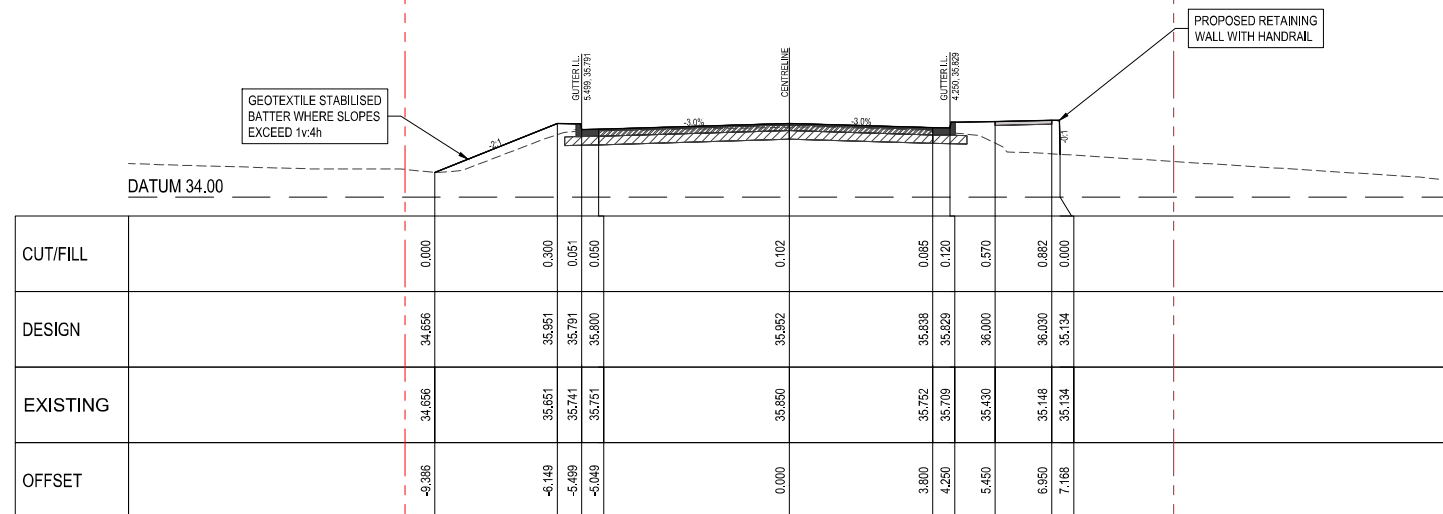
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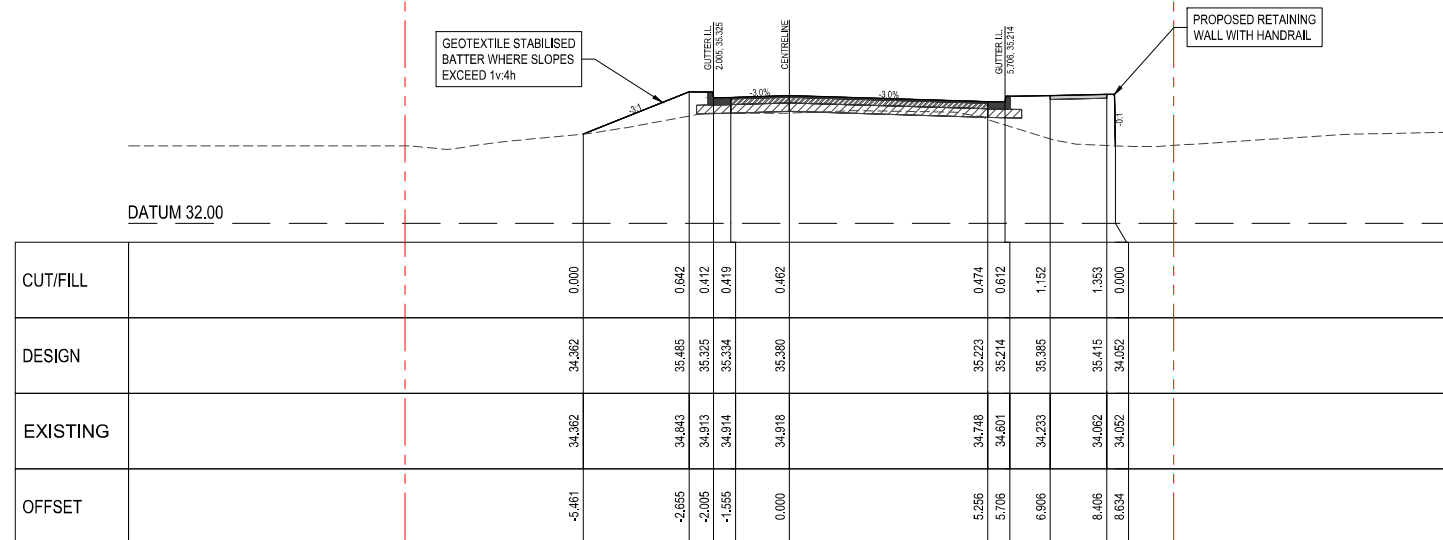
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CH 45.000



CH 60.000



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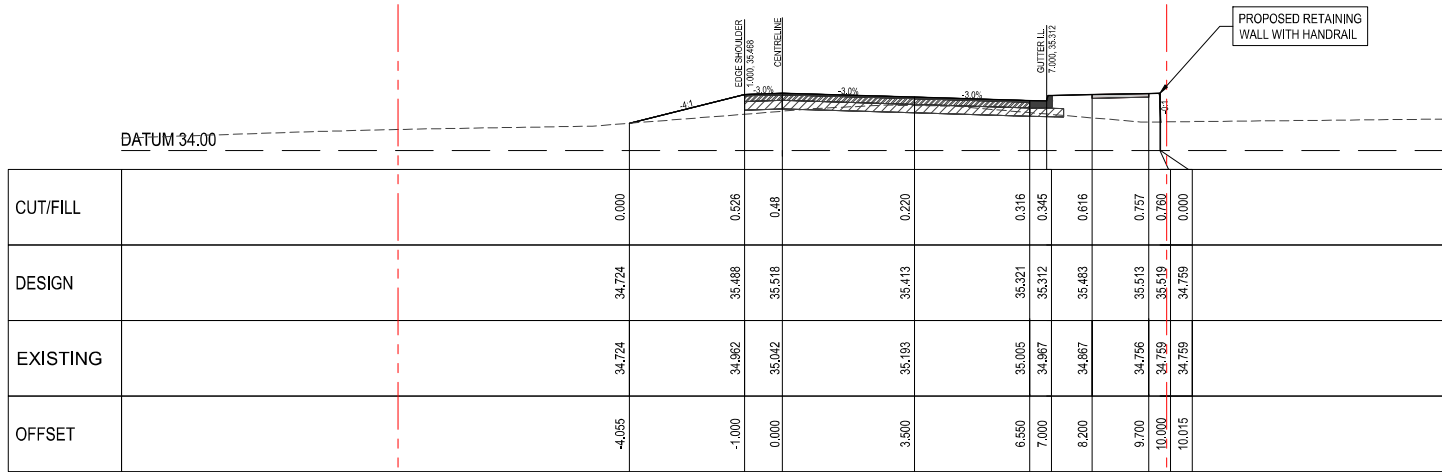
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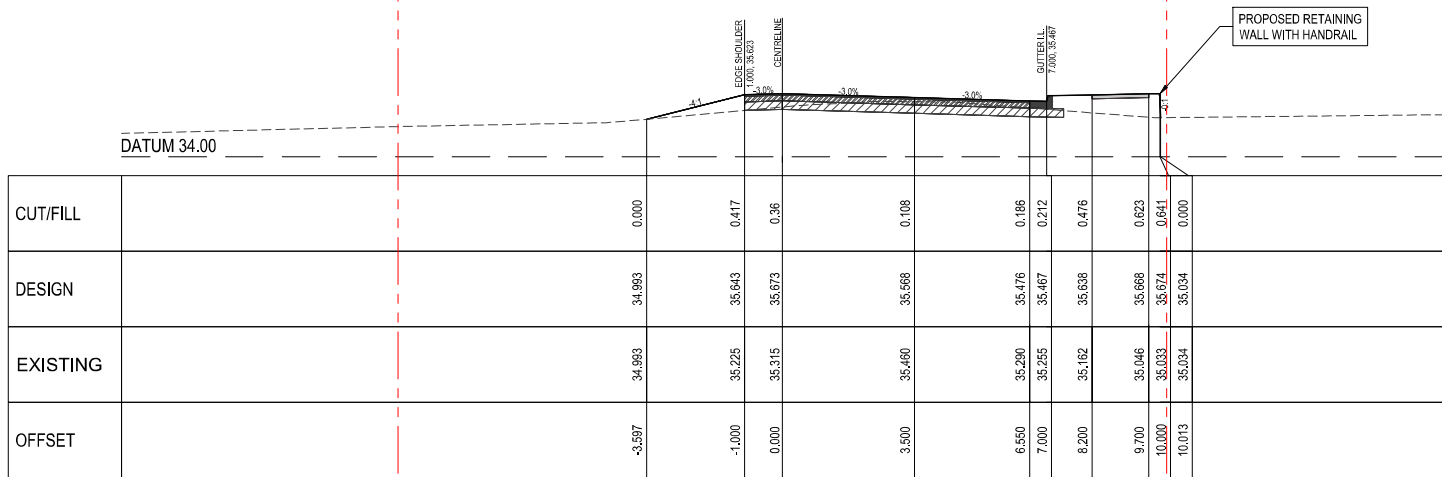
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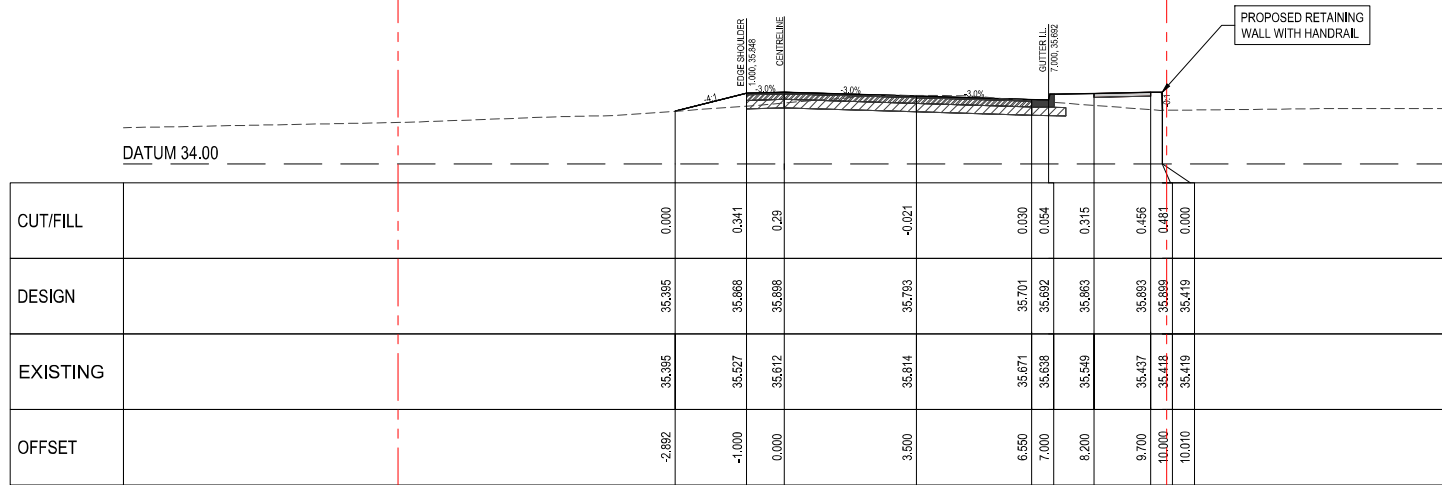
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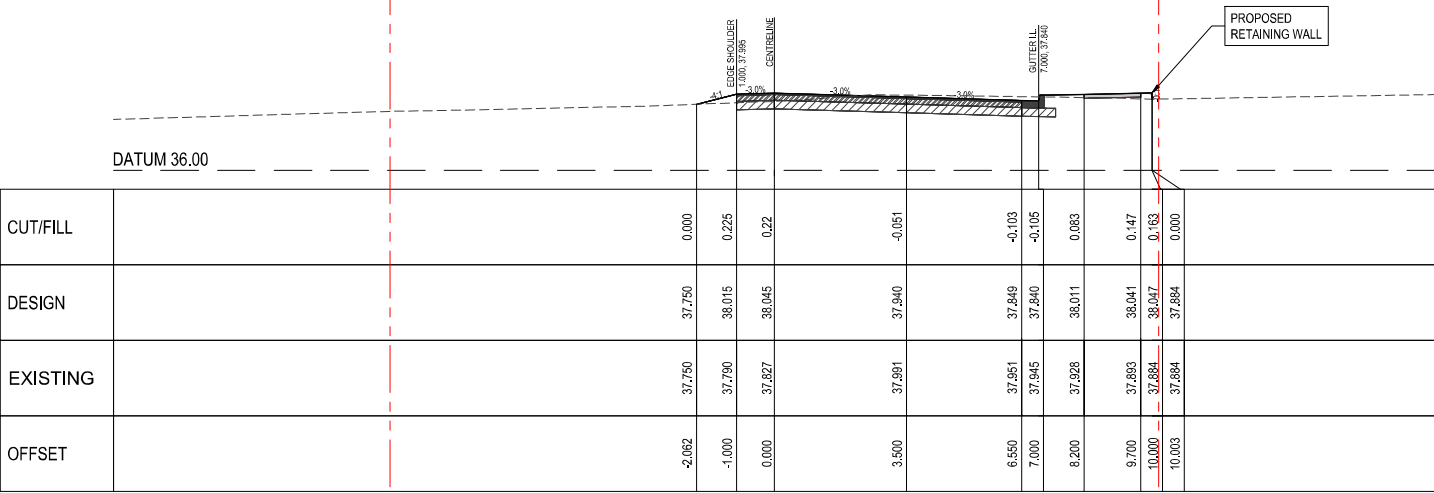
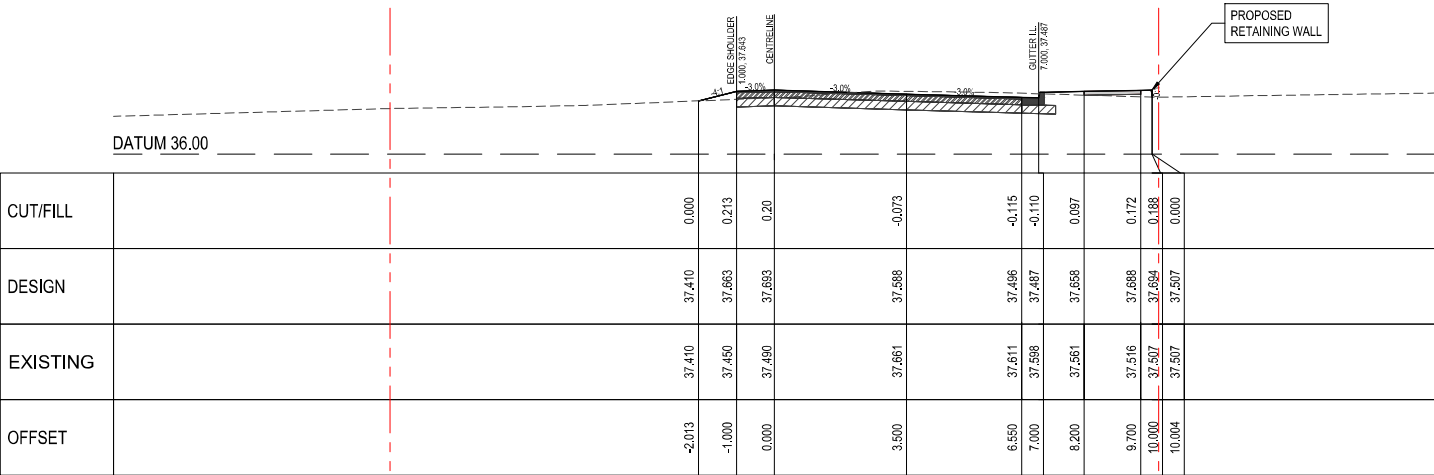
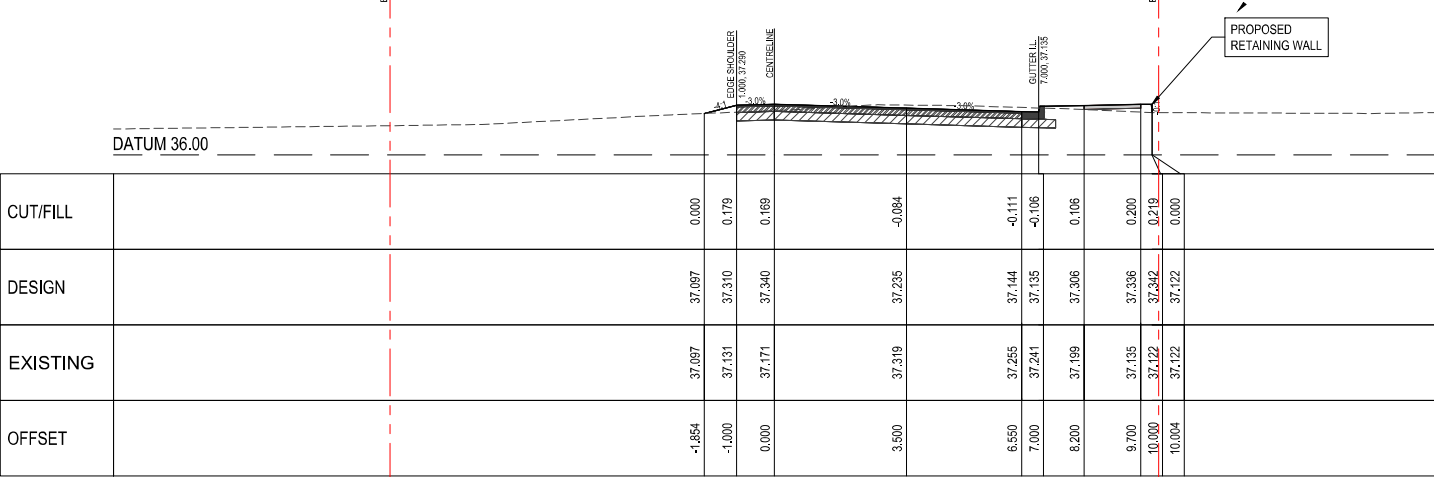
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WYNDELLA ROAD  
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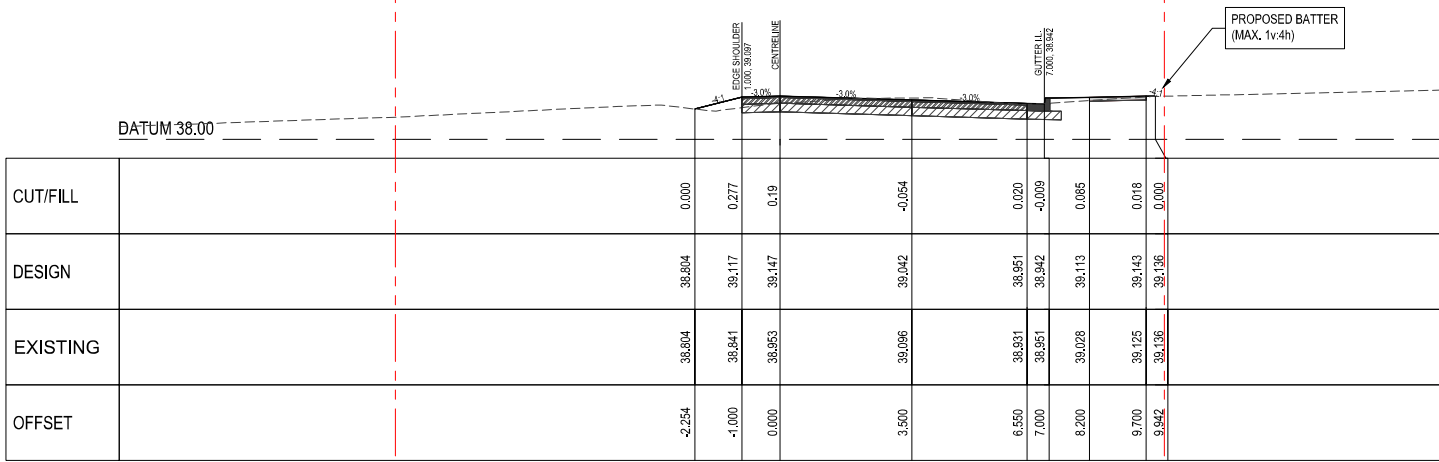
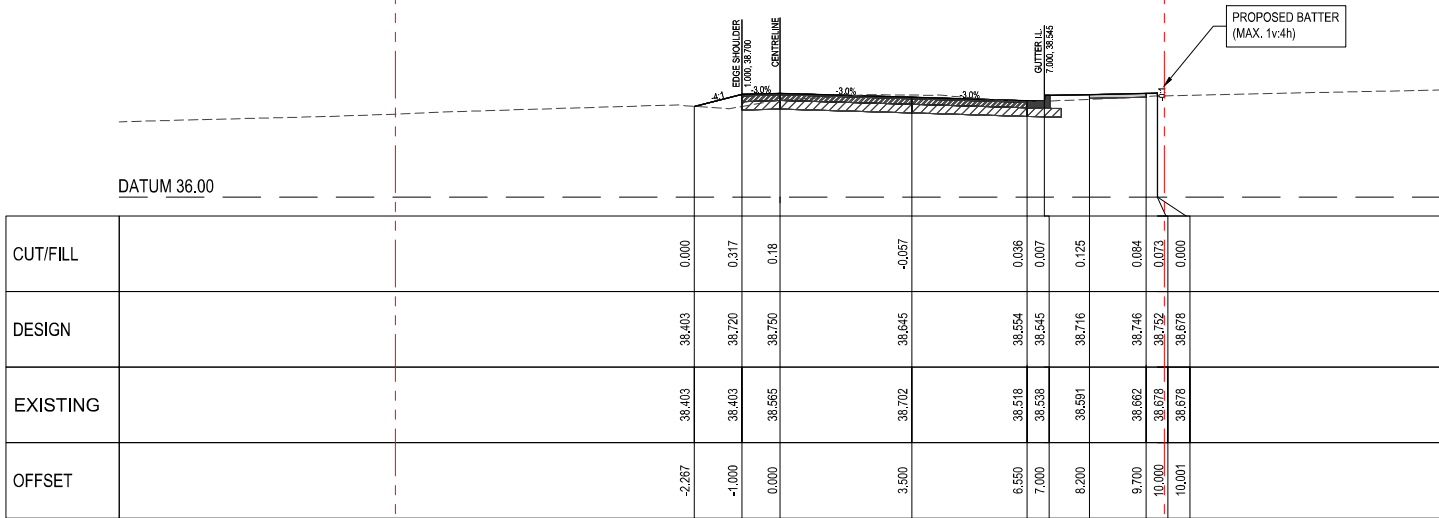
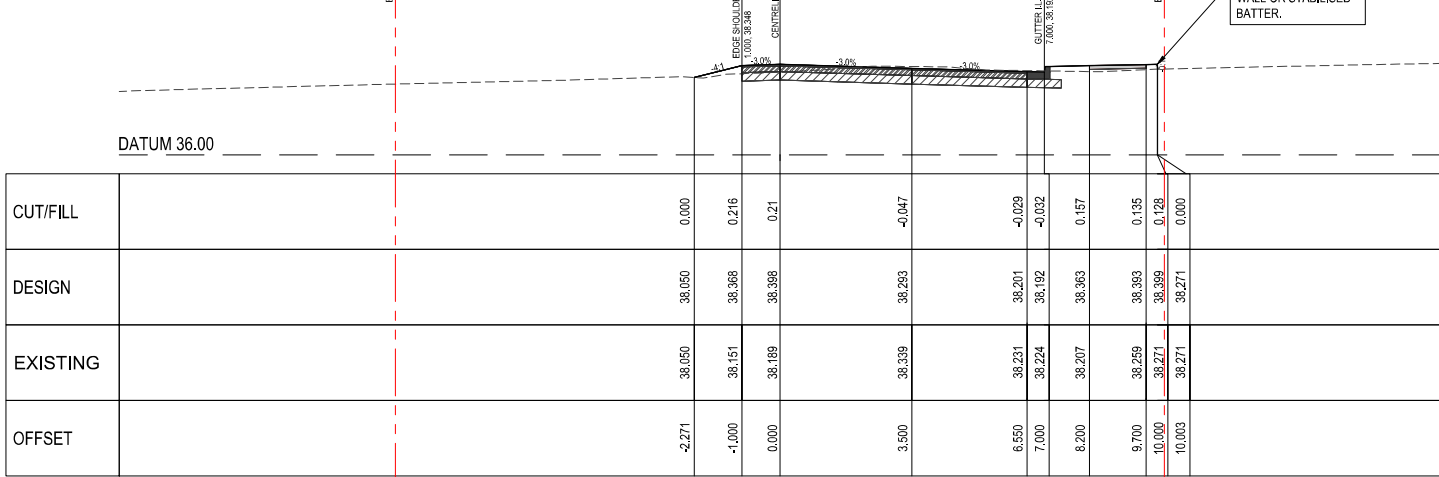


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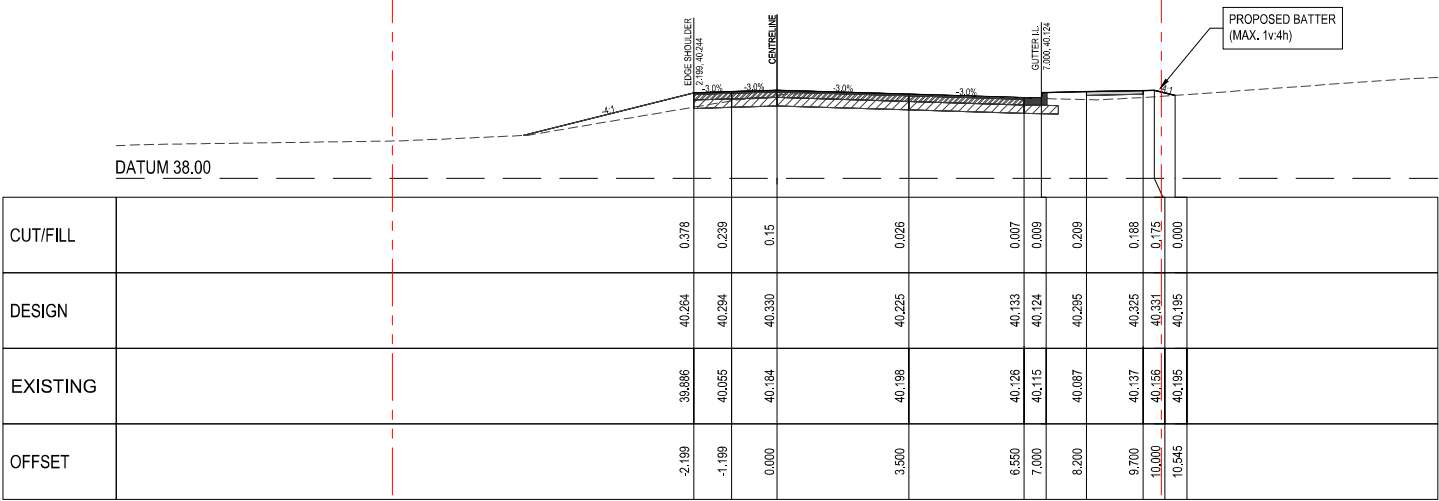
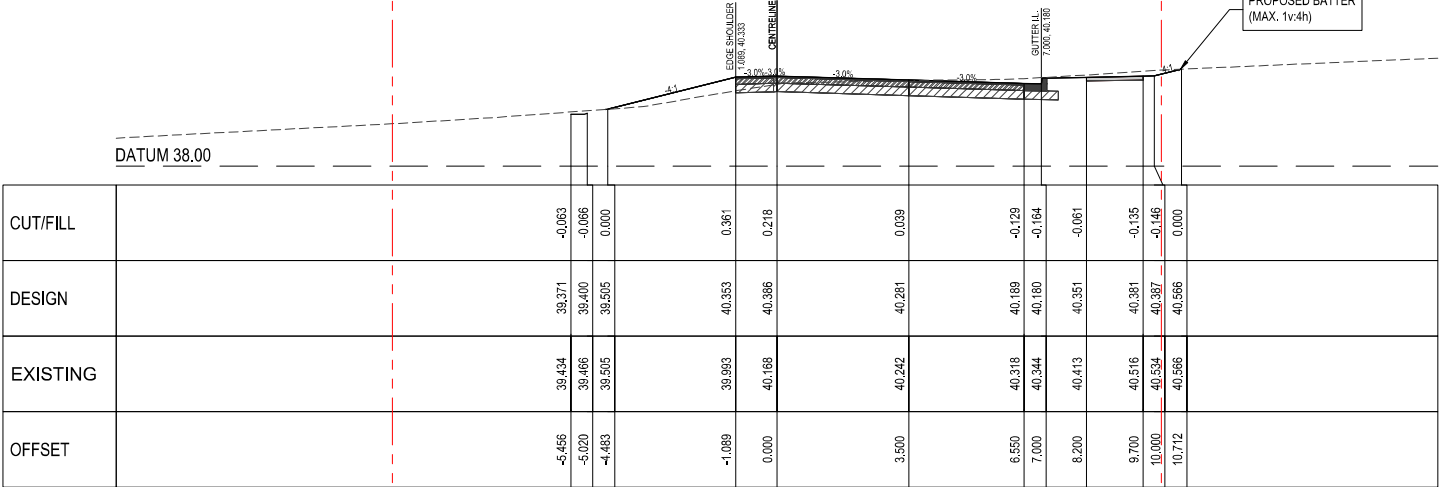
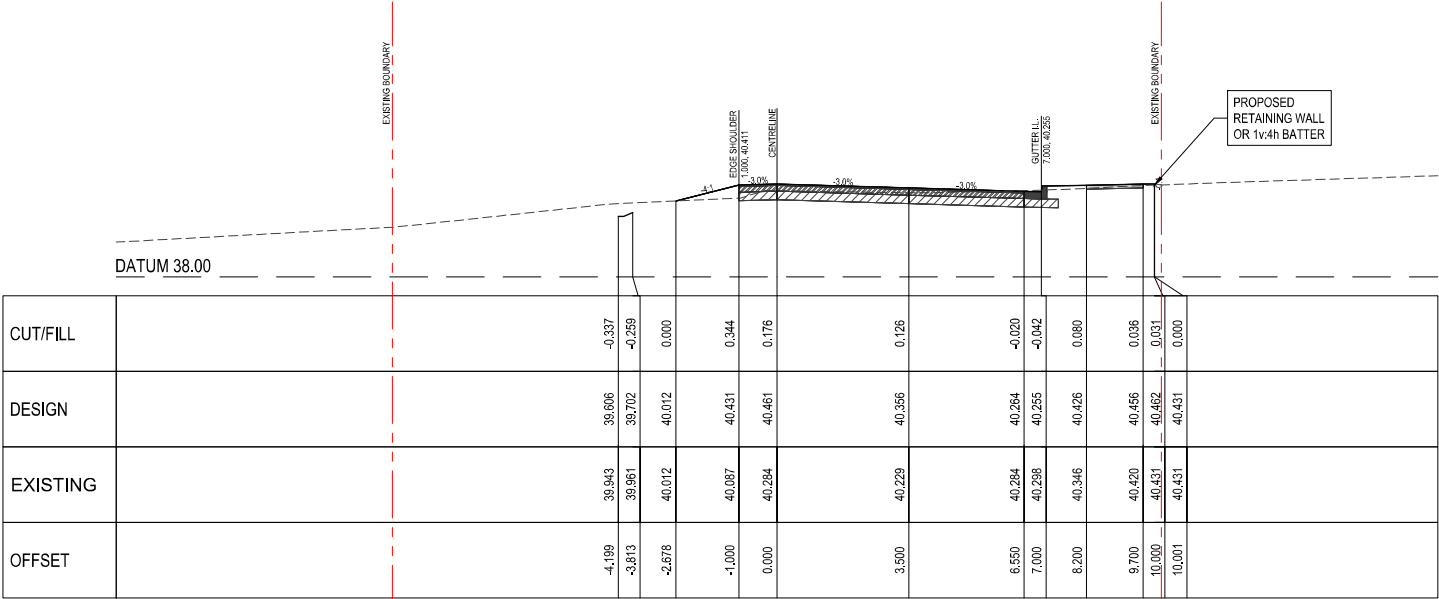
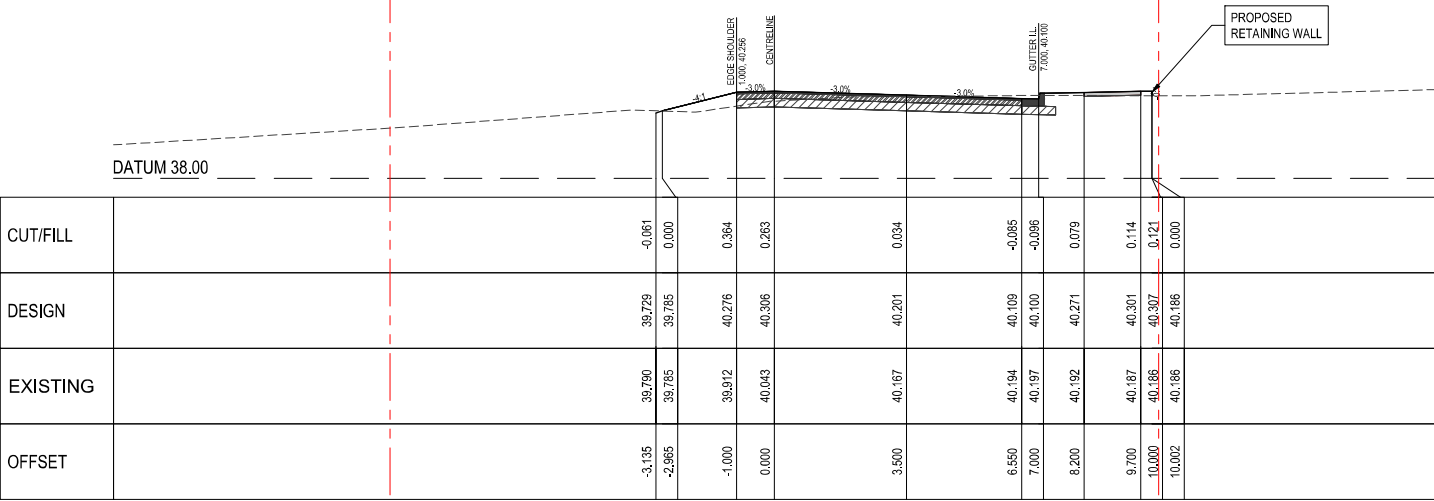
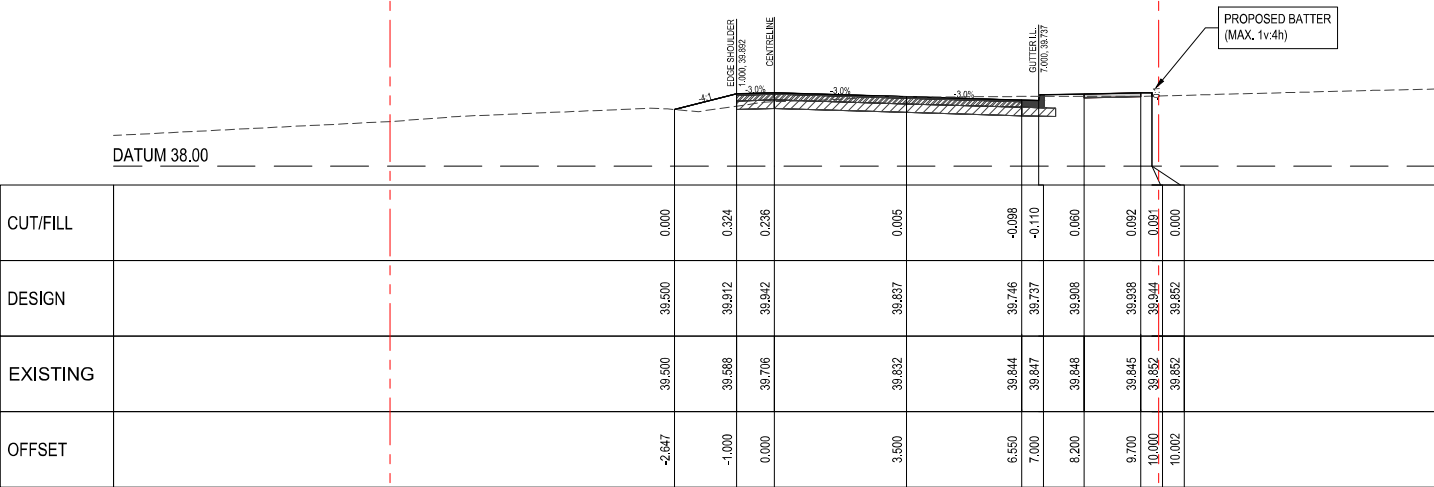
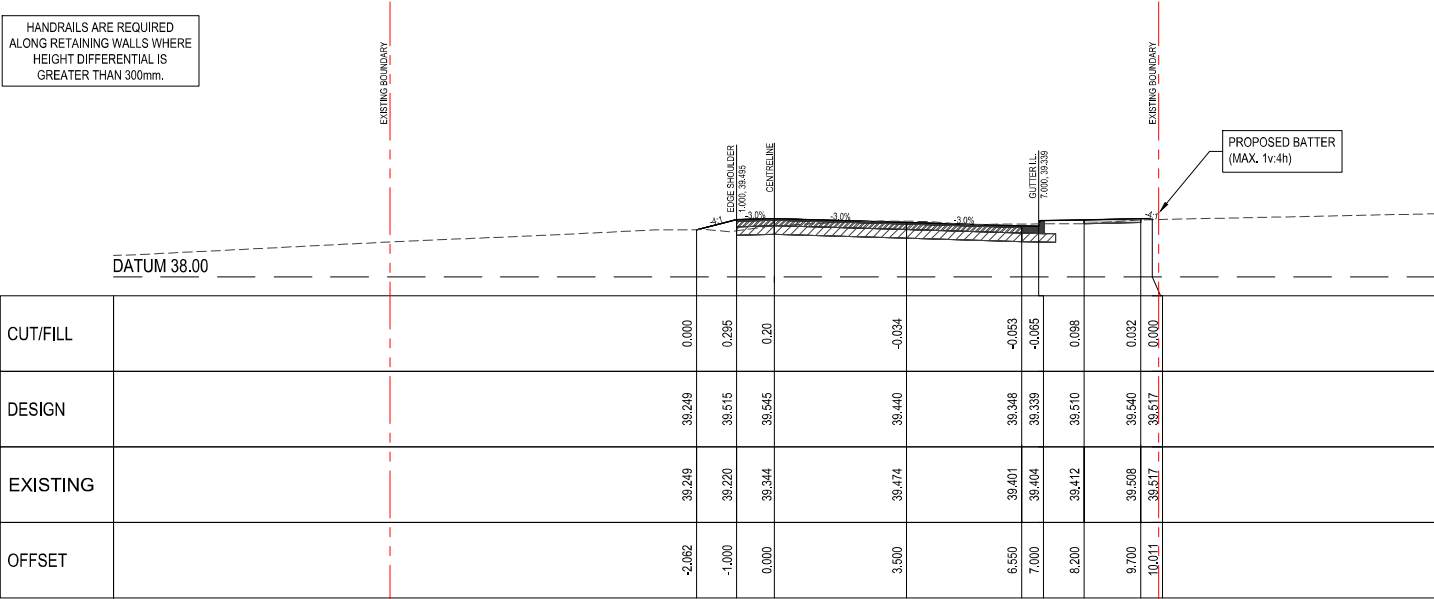
DRAWING TITLE:  
**CIVIL ROAD & SW PLANS**  
WYNDELLA ROAD  
CROSS-SECTIONS (3 OF 6)

DRAWING STATUS					
PRELIMINARY DESIGN					
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DRAWN	DESIGNED	CHECKED	APPROVED	ENDORSED	DATE
MS	KB	KB	KB	-	05.05.25
PROJECT No:		DRAWING No:			REV
23128		C04.05			B

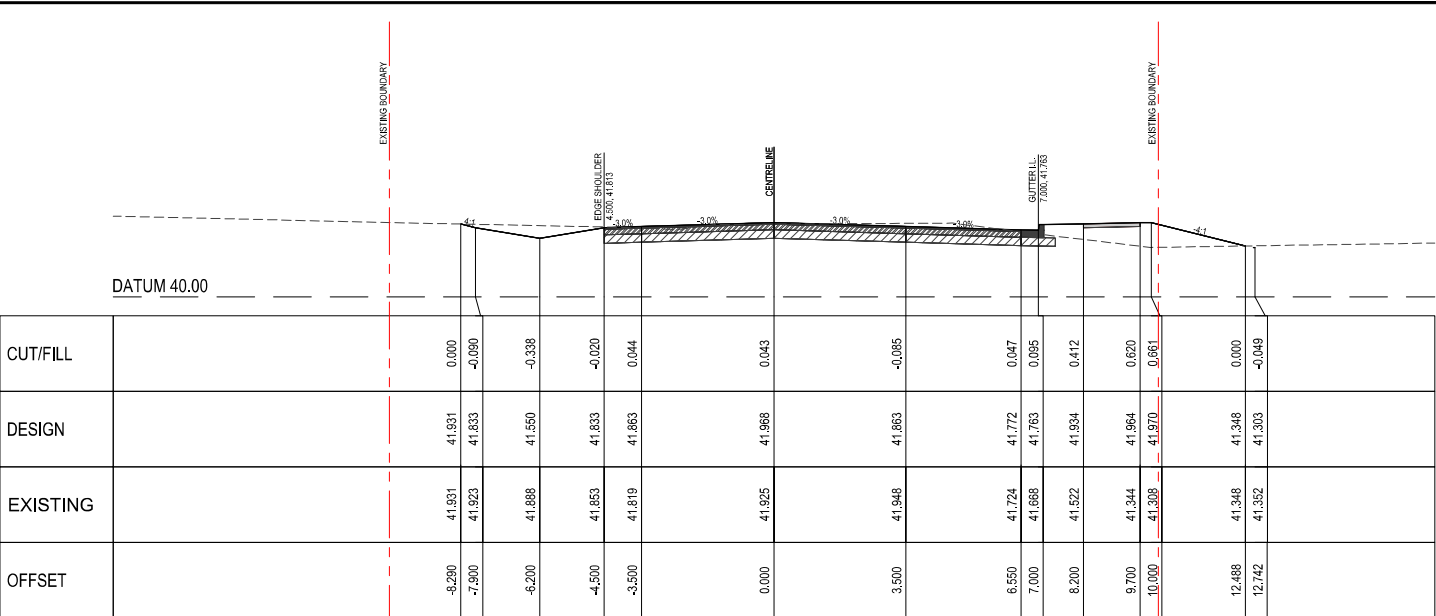




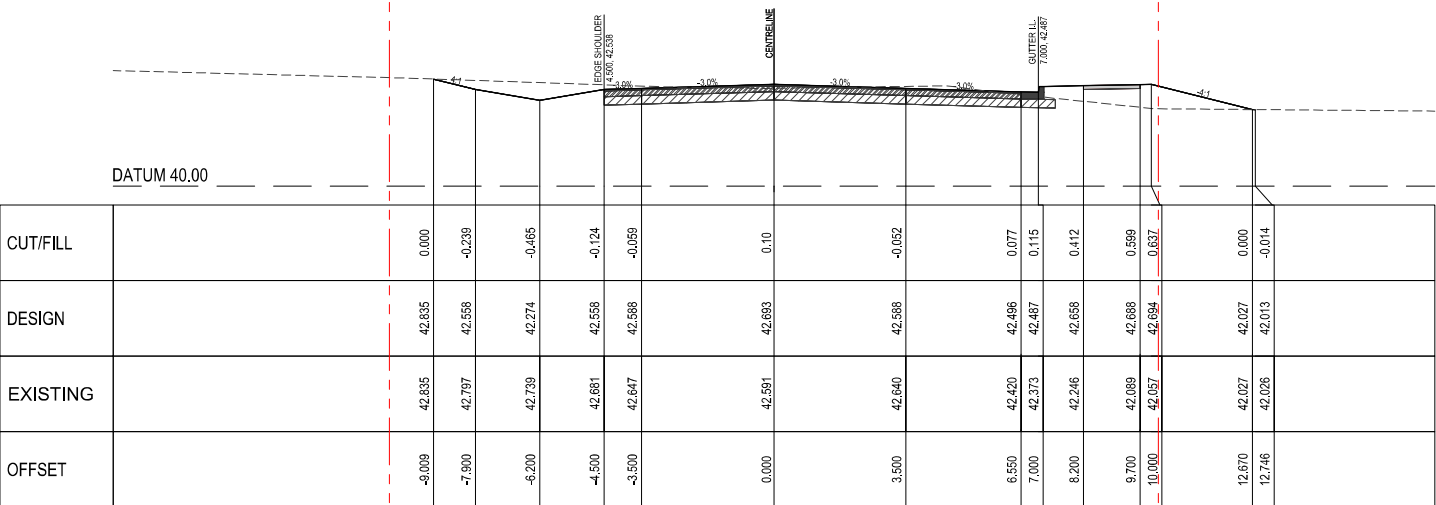
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ALONG RETAINING WALLS WHERE  
HEIGHT DIFFERENTIAL IS  
GREATER THAN 300mm.



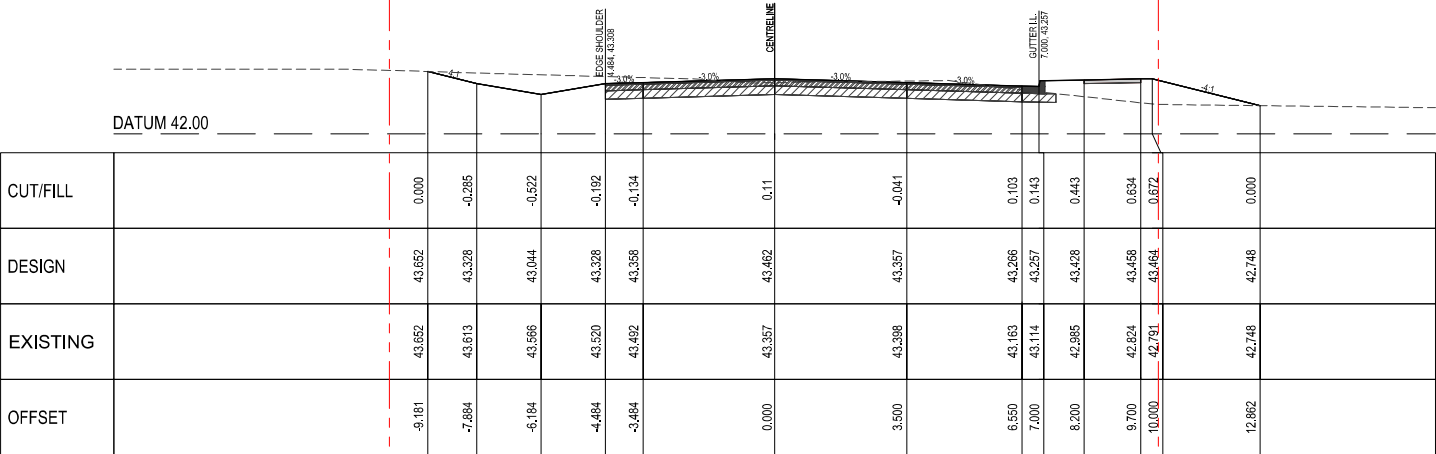




CH 435.000



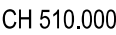
CH 450.000



CH 465.000

PROJECT No.	DRAWING No.	REV
23128	C04.07	B





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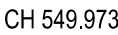
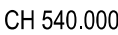
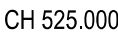
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phone: (02) 49 294 109  
email: [mail@wdsigroup.com.au](mailto:mail@wdsigroup.com.au)  
web: [www.wdsigroup.com.au](http://www.wdsigroup.com.au)

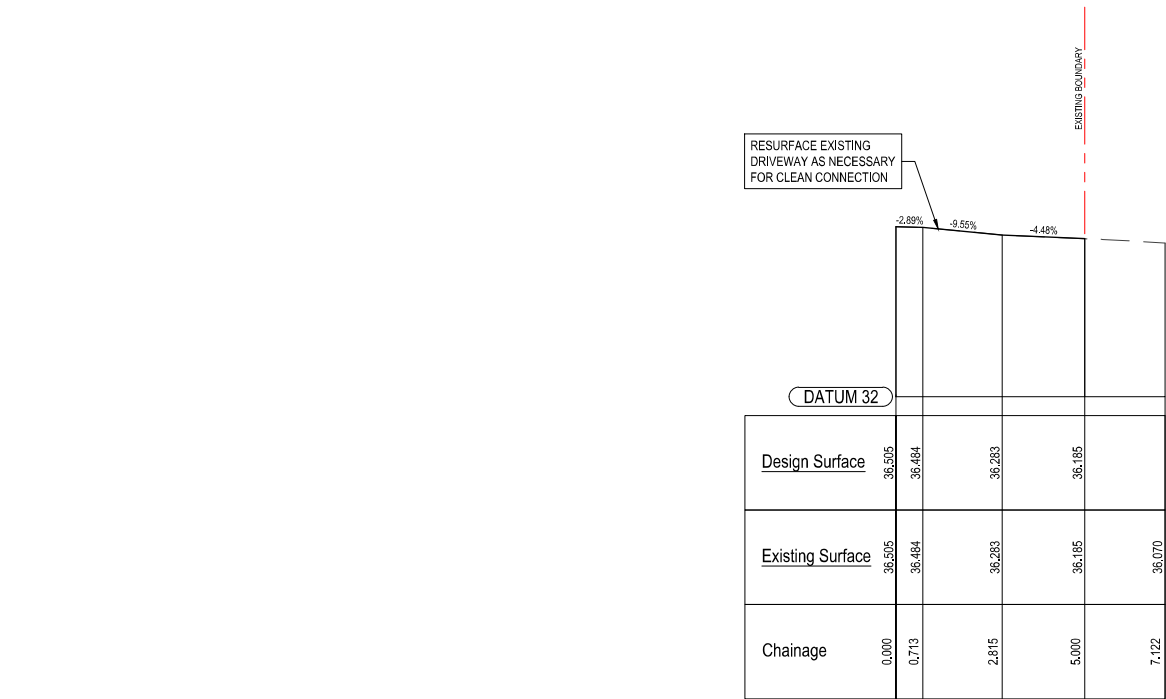
PROJECT:  
ROAD IMPROVEMENTS for WYNDELLA ROAD  
LOT 225, DP 246447  
34 WYNDELLA ROAD  
LOCHINVAR NSW 2321

DRAWING TITLE:  
**CIVIL ROAD & SW PLANS**  
WYNDELLA ROAD  
CROSS-SECTIONS (6 OF 6)

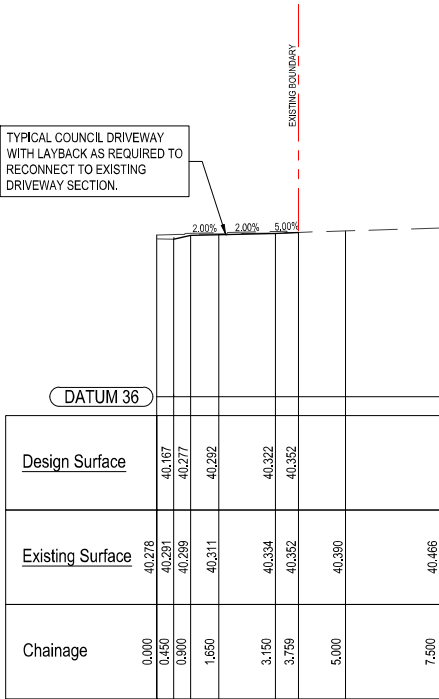
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PROJECT No. 23128		DRAWING No. C04.08			REV B







LONGITUDINAL SECTION  
LOT 223 DRIVEWAY



LONGITUDINAL SECTION  
LOT 224 DRIVEWAY



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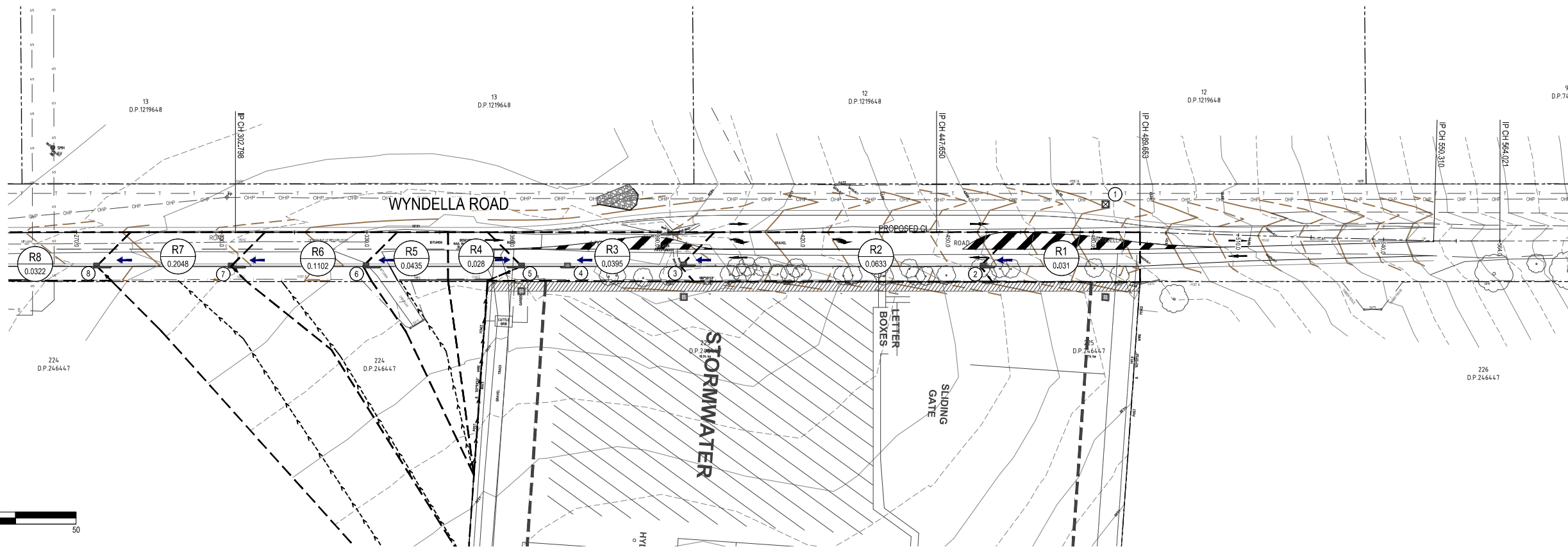
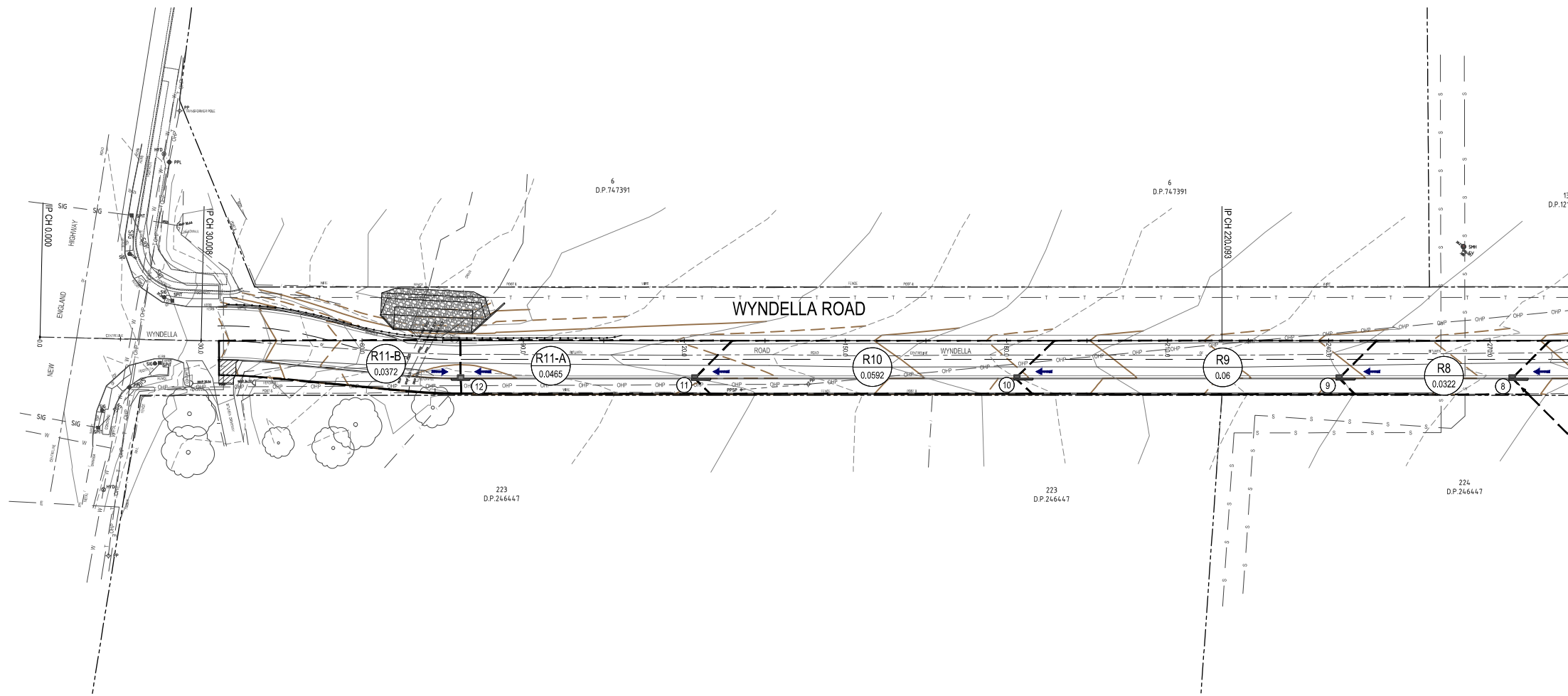
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PROJECT:  
ROAD IMPROVEMENTS for WYNDELLA ROAD  
LOT 225, DP 246447  
34 WYNDELLA ROAD  
LOCHINVAR NSW 2321

DRAWING TITLE:  
**CIVIL ROAD & SW PLANS**  
LOT 223 AND 224 DRIVEWAY AND  
BUSHFIRE ACCESS  
LONGITUDINAL SECTIONS

DRAWING STATUS <b>PRELIMINARY DESIGN</b>					
SCALE: 1:100				ORIG. SIZE A1	
DRAWN MS	DESIGNED KB	CHECKED KB	APPROVED KB	ENDORSED -	DATE 05.05.25
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PROJECT:  
ROAD IMPROVEMENTS for WYNDELLA ROAD  
LOT 225, DP 246447  
34 WYNDELLA ROAD  
LOCHINVAR NSW 2321

DRAWING TITLE:  
**CIVIL ROAD & SW PLANS**  
**STORMWATER CATCHMENT PLAN**

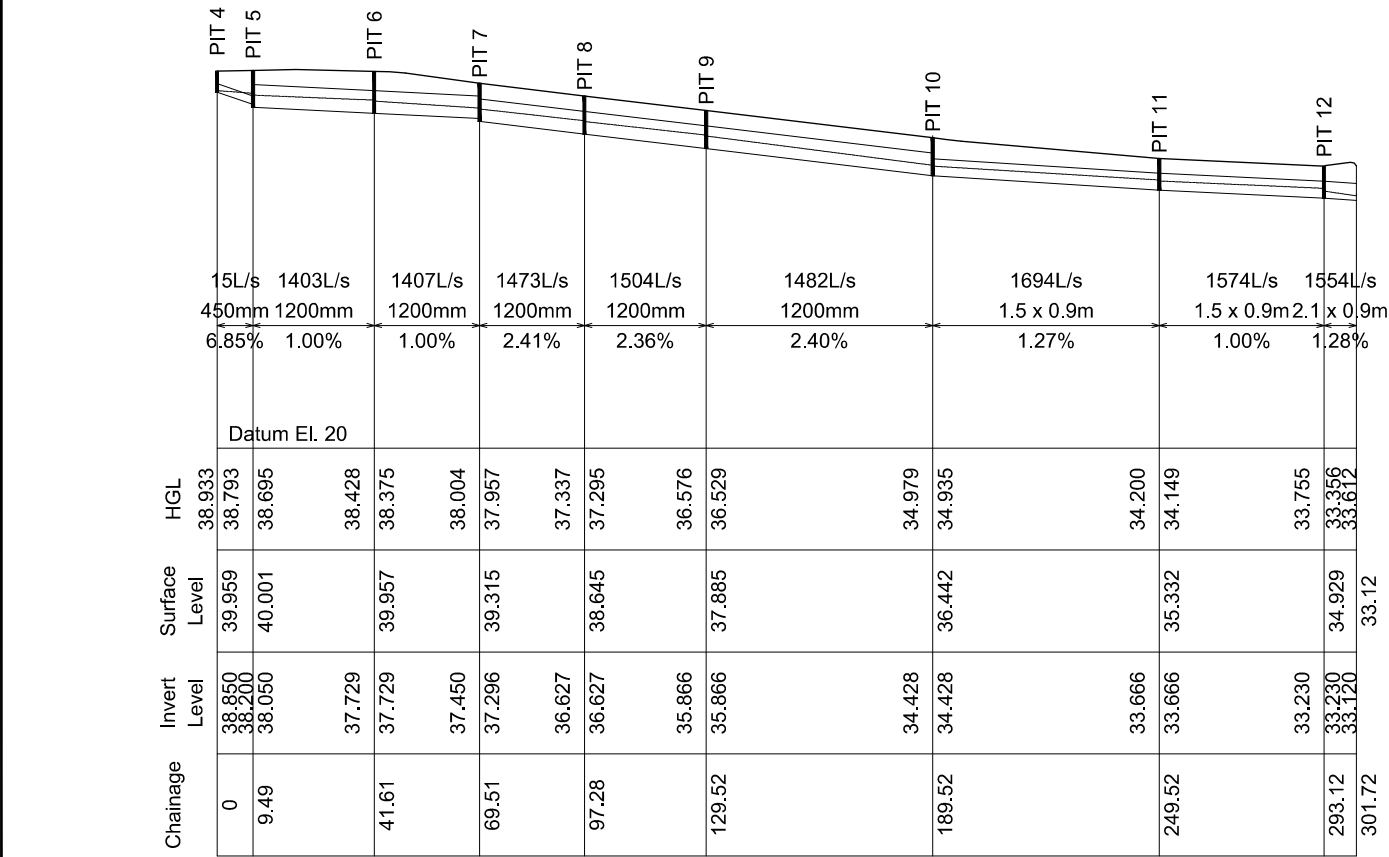
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PROJECT No: 23128		DRAWING No: C05.01				

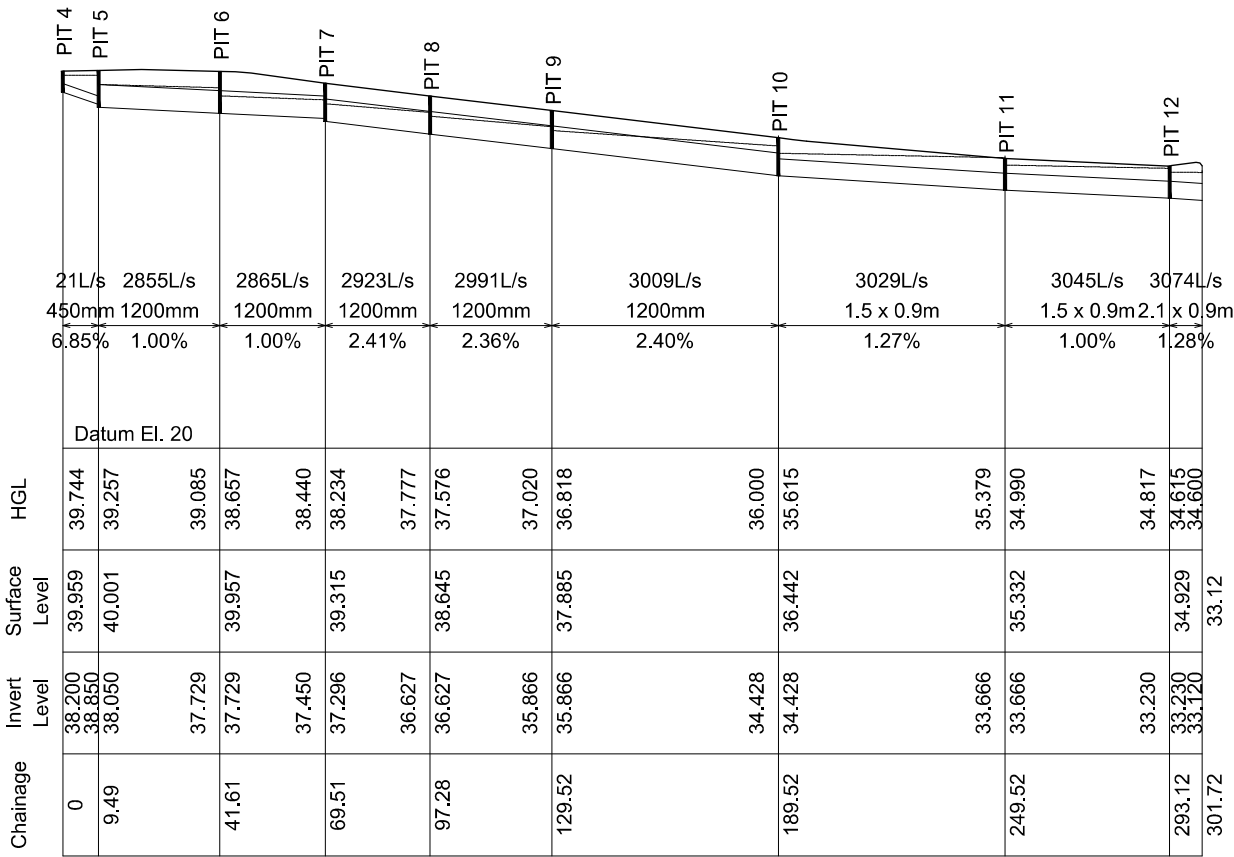




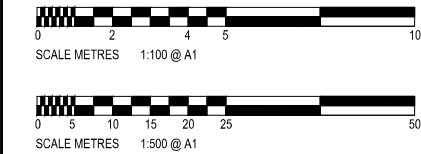




LINE 4 TO OUTFALL - 10%AEP  
1:500 (H): 1:100 (V)



LINE 4 TO OUTFALL - 1%AEP  
1:500 (H): 1:100 (V)



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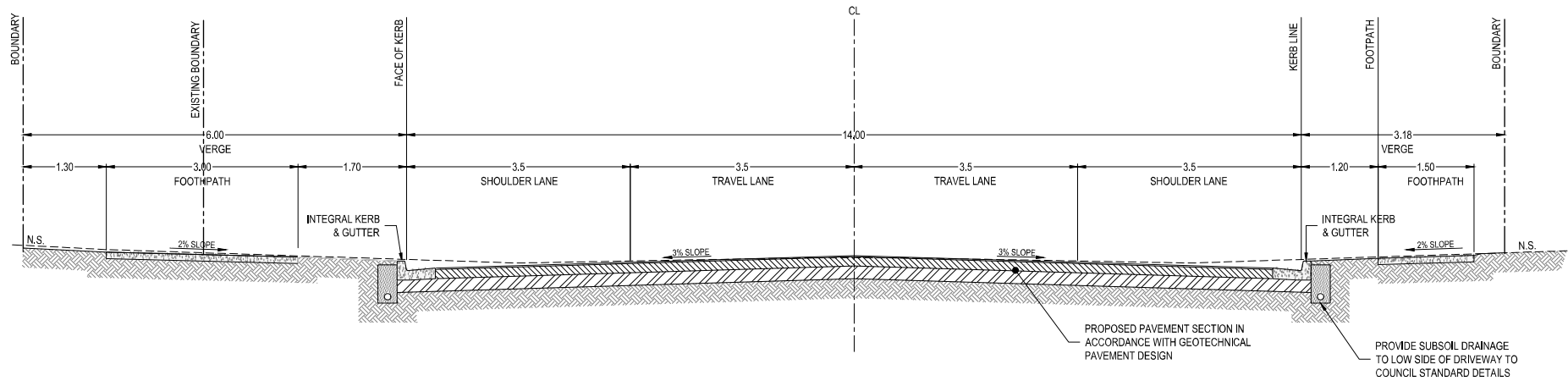
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ROAD IMPROVEMENTS for WYNDELLA ROAD  
LOT 225, DP 246447  
34 WYNDELLA ROAD  
LOCHINVAR NSW 2321

DRAWING TITLE:  
**CIVIL ROAD & SW PLANS**  
STORMWATER LONGITUDINAL SECTIONS

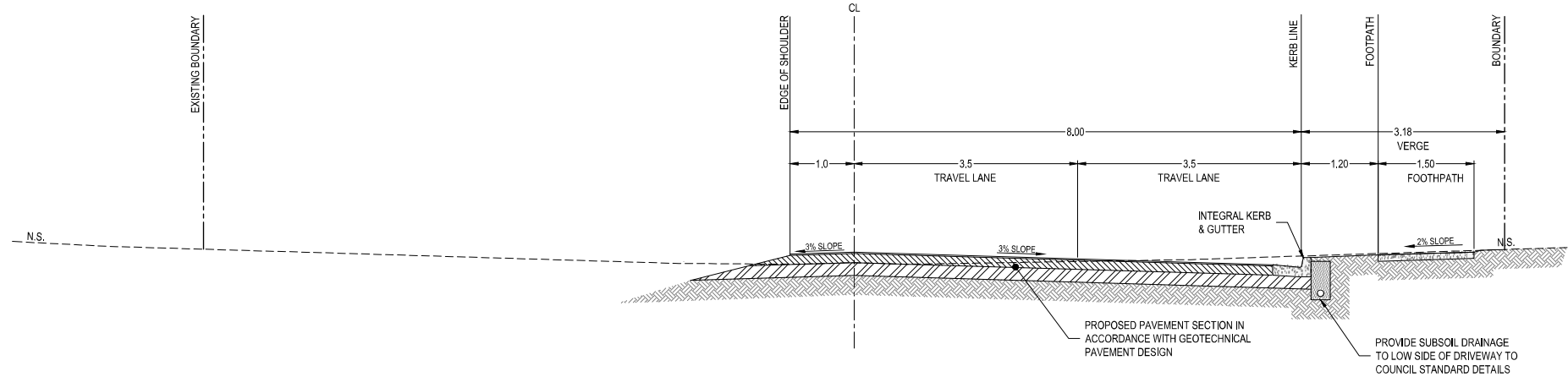
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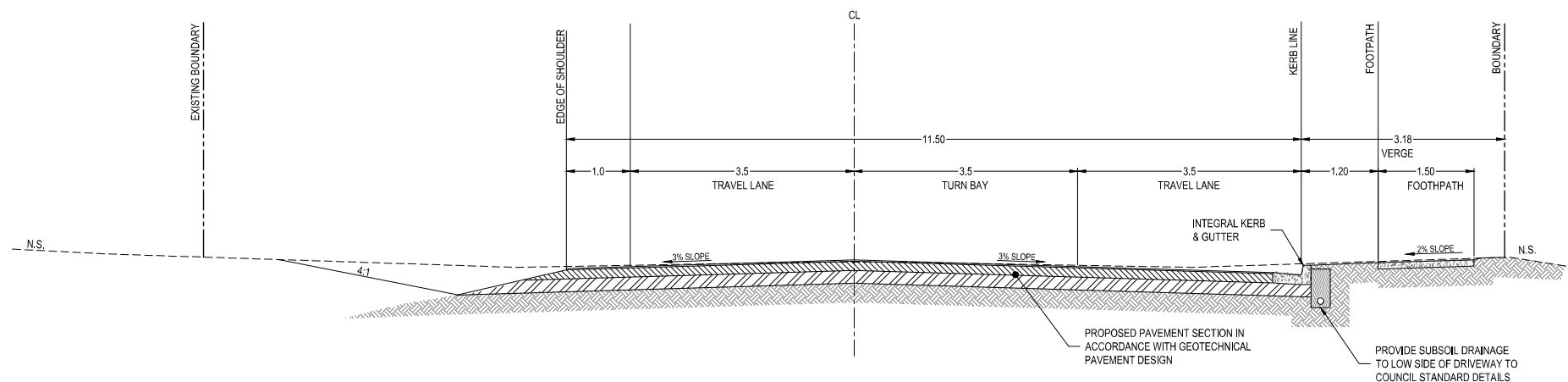
TYPICAL CROSS SECTION ULTIMATE WYNDELLA ROAD

1:50



TYPICAL CROSS SECTION WYNDELLA ROAD  
(SITE TO NEW ENGLAND HIGHWAY)

1:50



TYPICAL CROSS SECTION WYNDELLA ROAD  
(SITE FRONTAGE)

1:50



SCALE METRES 1:50 @ A1

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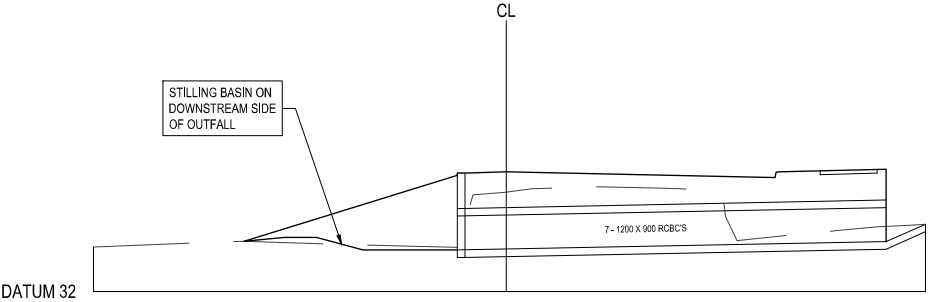
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34 WYNDELLA ROAD  
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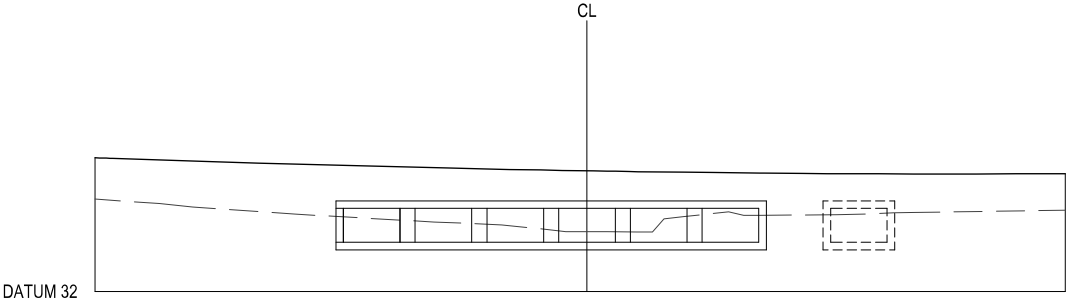
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**CIVIL ROAD & SW PLANS**  
TYPICAL ROAD SECTIONS

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PROJECT No: 23128		DRAWING No: C06.01				

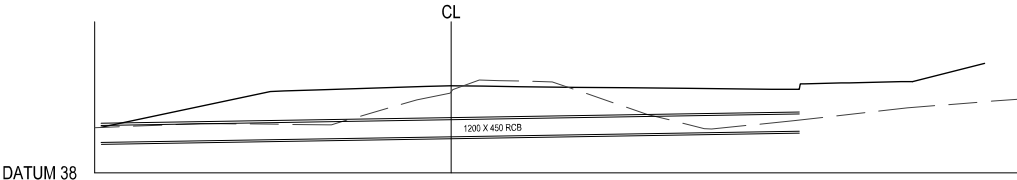




TYPICAL CULVERT No. 1 LONGITUDINAL SECTION



TYPICAL CULVERT No. 1 CROSS-SECTION  
INLET SIDE



TYPICAL CULVERT No. 1 LONGITUDINAL SECTION  
AT CENTRELINE OF CULVERT



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PROJECT:

ROAD IMPROVEMENTS for WYNDELLA ROAD  
LOT 225, DP 246447  
34 WYNDELLA ROAD  
LOCHINVAR NSW 2321

DRAWING TITLE:

**CIVIL ROAD & SW PLANS**  
TYPICAL CULVERT SECTIONS  
AND DETAILS

DRAWING STATUS **PRELIMINARY DESIGN**

SCALE:					ORIG. SIZE	
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DRAWN	DESIGNED	CHECKED	APPROVED	ENDORSED	DATE	REV
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23128		C06.02				