

ALTERATIONS & ADDITIONS TO EXISTING MEDICAL CENTRE
LOWER HUNTER MEDICAL CENTRE

LOTS 1 & 2 SECTION 2 IN DP 21143 269 – 271 NEW ENGLAND HIGHWAY, RUTHERFORD

PREPARED FOR: FLUIDITY SUPER FUND PTY LTD

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TRAFFIC & PARKING ASSESSMENT REPORT FLUIDITY SUPER FUND PTY LTD

ALTERATIONS AND ADDITIONS TO LOWER HUNTER MEDICAL CENTRE LOTS 1 & 2, SECTION 2 IN DP 21143 269 – 271 NEW ENGLAND HIGHWAY, RUTHERFORD.

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Date: - 20th September 2022

d. Garry

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summary

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1. INTRODUCTION

Intersect Traffic Pty Ltd (Intersect Traffic) has been engaged by Skelcon Pty Ltd on behalf of Fluidity Super Fund Pty Ltd to undertake a traffic and parking assessment for alterations and additions to an existing medical centre known as the Lower Hunter Medical Centre on Lots 1 & 2 Section 2 in DP 21143, 269 – 271 New England Highway, Rutherford. The site currently contains a converted dwelling operating as the Lower Hunter Medical Centre at 271 New England Highway and an existing residence on 269 New England Highway. The development proposes alterations to the two existing buildings on the site as well as connection of the two buildings and the extension / reconfiguring of the existing car parking area servicing the existing medical centre. The concept plans for the development are provided in **Attachment A.**

This traffic and parking assessment is required to support a development application to Maitland City Council seeking approval for the proposed development. The purpose of this document is to undertake an assessment of the likely traffic and parking impacts of the proposal on the local and state road network and associated roadside infrastructure. It will allow Maitland City Council (Council) to assess the merits of the application from a traffic impact perspective.

This report presents the findings of the traffic and parking assessment and includes the following:

- An outline of the existing situation in the vicinity of the site.
- An assessment of the traffic impacts of the proposed development including the predicted traffic generation and its impact on existing road and intersection capacities.
- Reviews the on-site parking provided within the proposed development and assesses it against Council and Australian Standards requirements.
- Presentation of conclusions and recommendations.



2. DEVELOPMENT PROPOSAL

2.1 Site Description

The site is located on the south-western side of the New England Highway, Rutherford immediately south of Arthur Street. It is adjacent to the Rutherford Shopping Precinct which is centred approximately 250 metres south of the shopping precinct and approximately 3.4 km's north-west of the Maitland CBD. The site is shown in the context of the surrounding areas in *Figure 1* below.



Figure 1 - Site Location

The site has the following property descriptors:

- Lots 1 & 2, Section 2 in DP 21143,
- Street address of 269 271 New England Highway, Rutherford,
- Total development site area of approximately 1,189 m², and
- Land zoning of R1 General Residential pursuant to the Maitland LEP (MLEP 2011).

The site currently has an existing single width residential vehicular access off the New England Highway approximately 20 metres south of Arthur Street servicing the residential dwelling at 269 New England Highway and two single width concrete residential accesses off Arthur Street 26 metres and 36 metres west of the New England Highway (10 metre separation). *Photograph 1* below shows the site from the New England Highway including the existing vehicular access crossing off the Highway (269 New England Highway) and *Photograph 2* below show the site from Arthur Street including the existing vehicular access crossings to the site (271 New England Highway).





Photograph 1 – Existing site and vehicular access New England Highway



Photograph 2 – Existing site and vehicular access – Arthur Street



2.2 Development Proposal

The proposed development involves alterations and additions to an existing medical centre (Lower Hunter Medical Centre) at 271 New England Highway and an adjoining residential dwelling (269 New England Highway) including connection of the two buildings. A change of use of the residential dwelling to a medical centre will result in an expanded Lower Hunter medical Centre catering for more doctors and patients with expanded clinics and service.

- Alterations to the existing medical centre to increase the size of the waiting room and reception area as well as improved staff facilities and a consult room. This will include removal of the existing 2 car garage.
- Alterations and additions to the existing residential dwelling including provision of a first floor area and connection to the adjoining existing medical centre building to provide an additional 4 consult rooms, entry foyer, lift, stairs, training room, office space and amenities.
- Extended car parking area to provide a total of 15 on-site car parks including 3 stacked staff parking parks.
- Modification of the existing Arthur Street access to the site providing a 6 metre wide combined entry / exit access to the site and removal of the existing residential access off the New England Highway.
- Property drainage to Maitland City Council's requirements; and
- Landscaping.

The development concept plans are shown in *Attachment A*.

It is understood the expansion of the Lower Hunter Medical Centre will increase staff on the site to a maximum of 4 practitioners and 8 other staff on-site from the current maximum of 2 practitioners and 5 staff on the site.

2.3 Existing Road Network

2.3.1 - New England Highway

The New England Highway near the site is an arterial road and is known as Highway HW 9 being a classified state highway. It is therefore under the care and control of TfNSW. However with the construction and operation of the nearby Hunter Expressway this section of New England Road now operates principally as a major collector road in the Maitland / Rutherford area directing local traffic to retail, industrial, medical and educational destinations in the area.

Near the site the New England Highway is a sealed four lane two way divided urban road with generally two travel lanes and a breakdown / parking lane in each direction except where additional lanes are provided for widening at intersections. Travel lanes vary between 3.2 to 3.5 metres in width with parking / breakdown lanes approximately 2.7 metres in width. The road is centre line and edge line marked and is signposted with a 60 km/h speed limit near the site.

Photograph 3 below shows the standard of the New England Highway near the site. At the time of inspection, the New England Highway was observed to be in good condition.

2.3.2 - Arthur Street

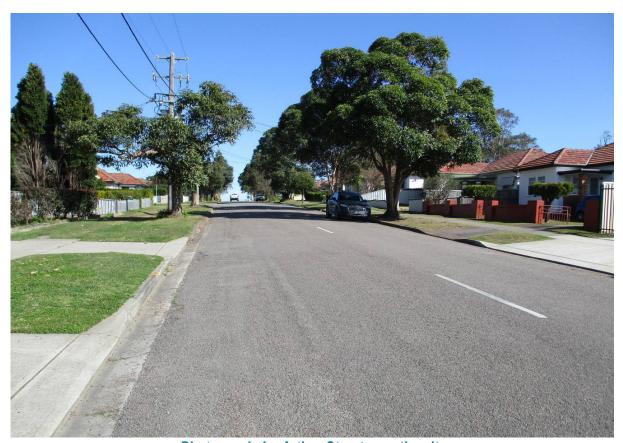
Arthur Street near the site is a sealed urban local road with its primary function to provide vehicular access to properties along its length. It is therefore under the care and control of Maitland City Council. It has kerb and gutter and with a carriageway width of approximately 9 metres allows a single travel lane in each direction and some on-street car parking.

Arthur Street was observed to be in fair condition as shown in **Photograph 4** below and a 50 km/h speed zoning applies to Arthur Street. The New England Highway / Arthur Street intersection is a signalised four way intersection with pedestrian crossing phases on both Arthur Street legs and the southern Highway leg as shown in **Photograph 5** below.



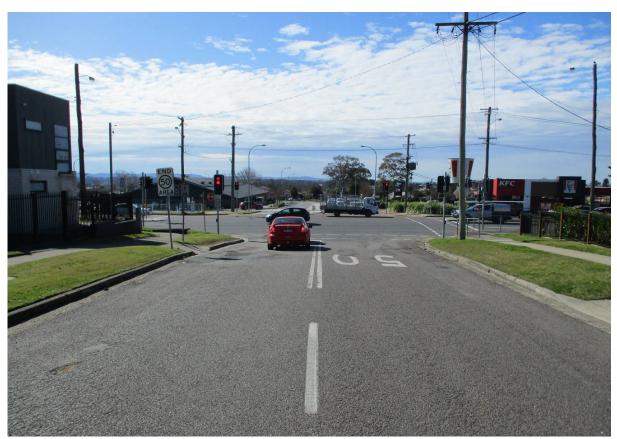


Photograph 3 – New England Highway near the site



Photograph 4 – Arthur Street near the site





Photograph 5 – New England Highway / Arthur Street signalised intersection

2.4 Existing Traffic

Existing traffic data for the intersection has been sourced from Northern Transport Planning and Engineering who undertook counts at the intersection in March 2017. This is the most recent data available and has been used in this assessment. The count identified the peak traffic periods as 8 am -9 am and 4.30 pm -5.30 pm which will coincide with likely peak traffic generation periods for the development. The count tally sheets are shown in **Attachment B** while the peak two-way midblock traffic volumes used in this assessment based on these counts are shown below in **Table 1**. Note current (2022) and future traffic (2022) have been estimated using a background traffic growth rate of 1.5 % per annum as recommended for use in the Lower Hunter area by TfNSW.

Table 1 – Existing and Future Peak Hour Traffic Volumes

		2022		2032 @ 1.5%	
Road	Section	AM (vtph)	PM (vtph)	AM (vtph)	PM (vtph)
New England Highway	south of Arthur Street	2563	2950	2974	3423
New England Highway	north of Arthur Street	2188	2395	2539	2779
Arthur Street	east of New England Highway	607	933	704	1083
Arthur Street	west of New England Highway	76	107	89	124



2.5 Traffic Generation

General guidelines on traffic generation are provided within TfNSW's *RTA's Guide to Traffic Generating Developments*. The applicable rate for the proposed development, taken from the Issue 2.2, October 2002 of the *Guide to Traffic Generating Developments*, are:

Extended Hours Medical Centre

Whilst the development is not an extended hours medical centre, rather being a specialists medical centre traffic generation from the site would be expected to be lower than that for an extended hours medical centre. However, adoption of the extended hours medical centre rates ensures a robust, worst case traffic impact assessment is carried out. The relevant AM and PM peak traffic generation rates for an extended hours medical centre are as follows;

```
AM peak = 10.4 vehicles/hr/100 m<sup>2</sup> gross floor area; and PM peak = 8.8 vehicles/hr/100 m<sup>2</sup> gross floor area
```

Therefore noting the additional medical centre GFA is approximately 350 m² GFA and the existing dwelling on the site which is to have a change of use would have generated approximately 1 vtph in both the AM and PM peaks the additional traffic generation from the proposed development can be calculated as follows (rounded up);

```
AM peak = 350/100 \times 10.4 - 1 = 36 \text{ vtph}; and PM peak = 350/100 \times 8.8 - 1 = 30 \text{ vtph}.
```

This traffic associated with the development needs to be distributed through the road network and the likely traffic distribution assumptions adopted for this assessment are:

- > 50 % of traffic will enter the site and 50 % of traffic will exit the site in a peak hour,
- ➤ The traffic entering / exiting the site will have origins / destinations:
 - New England Highway north 30 %.
 - New England Highway south 40 %.
 - Arthur Street east 20 %; and
 - Arthur Street west 10 %.

The resulting trip distribution adopted in this assessment is presented diagrammatically in *Figure 2* below.

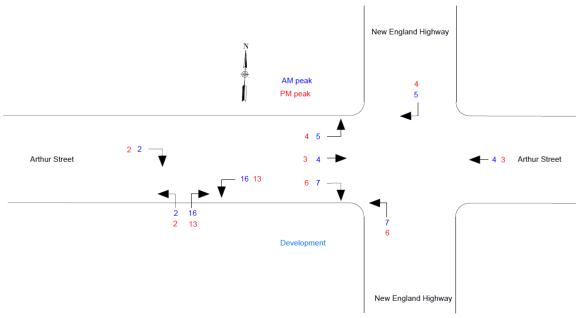


Figure 2 – Development Trip Distributions



2.6 Traffic Impacts and Considerations

2.6.1 Road Network Capacity

The capacity of the road network is generally determined by the capacity of intersections. However, for urban roads *Table 4.3* of the *RTA's Guide to Traffic Generating Developments*, reproduced below, provides some guidance on mid-block capacities for a level of service (LoS) C.

Table 4.3

Typical mid-block capacities for urban roads with interrupted flow

Type of Road	One-Way Mid-block Lane Capacity (pcu/hr)				
Median or inner lane:	Divided Road	1,000			
Wedian of filler lane.	Undivided Road	900			
	With Adjacent Parking Lane	900			
Outer or kerb lane:	Clearway Conditions	900			
	Occasional Parked Cars	600			
4 lane undivided:	Occasional Parked Cars	1,500			
4 idile ullulviueu.	Clearway Conditions	1,800			
4 lane divided:	Clearway Conditions	1,900			

Source: - RTA's Guide to Traffic Generating Developments (2002).

Noting that the New England Highway is a four lane divided road with little on-street car parking then the one way capacity is considered to be 1,800 vtph and the two way mid-block capacity is 3,600 vtph . Therefore, the adopted two-way mid-block capacity for the New England Highway is 3,600 vtph.

Arthur Street east of the New England Highway is a two lane two way road with no on-street car parking therefore has a one way capacity of 900 vtph and a two-way mid-block capacity of 1,800 vtph. Arthur Street west of the New England Highway is a two lane two way road with occasional parked cars therefore has a one way capacity of 600 vtph and a two way mid-block capacity of 1,200 vtph. These capacities for Arthur Street have been adopted in this assessment.

As existing traffic volumes on the New England Highway and Arthur Street shown in **Section 2.4**, are below the local road network two-way mid-block capacity determined above it is reasonable to conclude the local and state road network currently has spare capacity to cater for additional development in the area. When distributing the development traffic, as shown in **Figure 2**, the resulting maximum additional traffic on the New England Highway and Arthur Street is not sufficient for the local and state road network to reach its two-way mid-block capacities as shown in **Table 2** below.

Table 2 – Capacity and two-way traffic mid-block peak hour traffic volumes post-development

		Capacity	2022 + development		2032 + development		t Development traffic	
Road	Section	vtph	AM (vtph)	PM (vtph)	AM (vtph)	PM (vtph)	AM	PM
New England Highway	south of Arthur Street	3,600	2577	2962	2988	3435	14	12
New England Highway	north of Arthur Street	3,600	2198	2403	2549	2787	10	8
Arthur Street	east of New England Highway	1,800	615	939	712	1089	8	6
Arthur Street	west of New England Highway	1,200	80	111	93	128	4	4

Therefore, it is reasonable to conclude that the development will not adversely impact on the levels of service experienced on the state and local road network near the site.



2.6.2 Intersection Capacity

The major intersection likely to be impacted by this development is the New England Highway / Arthur Street signalised cross intersection. As the traffic gets distributed further through the road network the impact of the development becomes significantly less and they would be expected to easily cater for any additional traffic generated by the development.

To analyse the impact of the additional traffic from the development on the New England Highway / Arthur Street intersection it has been modelled using the Sidra intersection modelling software. This micro-analytical program identifies "Level of Service" (LoS) criteria for intersection analysis which range from LoS A to LoS F. Assessment is then based on the level of service requirements of the RMS shown below.

Table 4.2 Level of service criteria for intersections

Level of Service	Average Delay per Vehicle (secs/veh)	Traffic Signals, Roundabout	Give Way & Stop Signs
Α	< 14	Good operation	Good operation
В	15 to 28	Good with acceptable delays & spare capacity	Acceptable delays & spare capacity
С	29 to 42	Satisfactory	Satisfactory, but accident study required
D	43 to 56	Operating near capacity	Near capacity & accident study required
E	57 to 70	At capacity; at signals, incidents will cause excessive delays	At capacity, requires other control mode
		Roundabouts require other control mode	

Source: - RTA's Guide to Traffic Generating Developments (2002)

The results of the modelling are summarised below in *Table 3* for the 'all vehicles' case and the full movement summary tables generated by the models are provided in *Attachment C*.

In undertaking this assessment, the following assumptions were made:

- The intersection layout is as constructed,
- 2022 existing traffic volumes is as per traffic counts in Attachment B with a 1.5 % per annum background traffic growth rate applied.
- 2032 traffic volumes were determined by applying a background traffic growth factor of 1.5 % p.a. on the road network.
- The model phasing was set to determine the Optimum Cycle Time to reduce overall delay at the intersection.
- The post development traffic distributions are as shown above in the Development Trip Distribution Diagram, *Figure 2* of *Section 2.5*.



Table 3 – New England Highway / Arthur Street signalised intersection – Sidra results summary

Modelled Peak	Cycle Time (s)	Degree of Saturation (v/c)	Worst Average Delay (s)	Worst Level of Service	95% back of queue length (cars)
2022 AM	75	0.793	26.1	В	18.7
2022 PM	120	0.953	53.4	D	47.7
2022 AM + development	75	0.793	26.3	В	18.9
2022 PM + development	120	0.953	53.7	D	48.1
2032 AM no development	80	0.9	38.6	С	31.5
2032 PM no development	150	1.136	155.8	F	111.2
2032 AM + development	80	0.904	39.1	С	31.6
2032 PM + development	150	1.137	157.0	F	112.1

The modelling for the New England Highway / Arthur Street signalised cross intersection shows the intersection will continue operate satisfactorily post development, in accordance with TfNSW criteria, with the increase in average delays and back of queue being minimal (less than 0.5 seconds and 0.4 vehicles, respectively). The modelling also shows that with background traffic growth only through to 2032, while the AM peak will continue to operate satisfactorily the PM peak will fail before 2032. This is an issue for the road authority as this will occur with just background traffic growth. The impact of the development on the operation of the intersection in 2032 however remains negligible with average delay and back of queue length increases being less than 1.2 seconds and 0.9 vehicles, respectively. Most importantly the development does not result in any deterioration in the overall LoS occurring at the intersection. Therefore, it is reasonable to conclude that the proposed development will not adversely impact on the efficiency and effectiveness of the local road network.

2.6.3 Site Access

To assess the performance of the Arthur Street access to the site, the table below, taken from Austroads *Guide to Traffic Management – Part 6 – Intersections, Interchanges & Crossings (2009)*, for which the guide states a detailed analysis to demonstrate adequate capacity is available is unlikely to be necessary as uninterrupted flow conditions would prevail, is utilised.

Major road type ¹	Major road flow (vph) ²	Minor road flow (vph) ³
	400	250
Two-lane	500	200
	650	100
	1000	100
Four-lane	1500	50
	2000	25

Notes:

- 1. Major road is through road (i.e. has priority).
- 2. Major road flow includes all major road traffic with priority over minor road traffic.
- 3. Minor road design volumes include through and turning volumes.

Source: - Austroads Guide to Traffic Management - Part 6 - Intersections, Interchanges & Crossings (2009)

It is noted that the 2032 peak hour mid-block two-way traffic, including development traffic, is likely to be a maximum of less than 128 vtph on Arthur Street and 36 vtph on the access. The traffic volumes predicted for the intersection are well below the lower volume amounts in the table above and would result in uninterrupted flow conditions. Therefore, it can be concluded that the modified Arthur Street access to the site will operate with uninterrupted flow conditions and an urban BAR / BAL treatment would be sufficient at the access (normal property access with no dedicated turning lanes).



The site access is proposed to service a car park with lower than 25 spaces. Under Table 3.1 of Australian Standard *AS2890.1-2004 Parking facilities — Part 1 - Off-street car parking* a car park with 0 - 25 spaces accessed via a local road providing short term parking (Class 3A) is required to have a Category 1 access facility. A Category 1 access facility is a combined entry / exit driveway between 3 metres and 5.5 metres wide. The proposal is to modify the existing separate entry and exit access to a combined entry / exit access 6 metres wide. Therefore the proposal is compliant with the Australian Standard *AS2890.1-2004 Parking facilities — Part 1 - Off-street car parking* requirement as this exceeds the requirements of a Category 1 access facility.

Under Figure 3.2 of Australian Standard *AS2890.1-2004 Parking facilities – Part 1 - Off-street car parking the sight distance* requirements for the access needs to be a minimum of 45 metres for a 50 km/h speed environment. The available sight distance at the proposed access to the car park to the site exceed this requirement therefore the access is also compliant with the Standard in this regard.

Therefore it is considered that the proposed access arrangements for the development car park are considered satisfactory and compliant with Council and Australian Standard requirements.

2.6.4 On-site parking and servicing

The Development Control Plan (2011) sets out the requirements for on-site parking to be provided for new developments in Section C11. However there are no rates contained in the DCP for medical centres. Therefore the default parking provision requirement is the RTA's Guide to Traffic Generating Developments which recommends a minimum parking rate of 1 space per 25 m² GFA for on-site car parking.

It is noted the existing medical centre provides a total of 5 on-site car spaces including a double garage. The additional medical centre floor area is approximately 350 m² GFA, however approximately 100 m² of this is a training room for staff which in effect is ancillary to the medical centre and will not generate additional car parking. Therefore the additional on-site car parking requirement for the development can be calculated as follows;

Additional on-site car parking = 250 / 25 = 10 spaces.

The total car parking requirement for the site therefore is as follows;

```
Required on-site car parking = existing parking + additional parking = 5 space + 10 spaces = 15 spaces.
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The plans currently show provision of 15 car spaces, including 3 stacked staff spaces along the southern side of the building. Therefore the on-site car parking provision is considered compliant with the *RTA's Guide to Traffic Generating Developments*. (RTA Guide.

The internal car park designs should comply with Australian Standard *AS2890.1-2004 Parking facilities – Part 1 – Off street car parking facilities.* Having reviewed the plans (scaling), it is considered the proposed car parking layout complies with the relevant Australian Standard for car spaces of a minimum of 5.4 metre long by 2.6 metre wide spaces with blind aisle extensions of 1.0 metre and aisle widths 5.8 metres for a User Class 3 (short term medical centre parking) carpark, however this should be confirmed at Construction Certificate stage.

The development is currently serviced via kerbside waste collection and generally light vehicles using available car parking. This is acceptable as servicing is generally a maximum of 1 per day or 2 or 3 per week. Overall, it is concluded that the proposed on-site car parking and servicing facilities can comply with Maitland City Council's DCP (2011) and Australian Standard AS2890.1-2004 Parking facilities – Part 1 – Off street car parking facilities as well as the RTA's Guide to Traffic Generating Developments (2002).



2.6.5 Alternative Transport Modes

Currently Hunter Valley Buses run public transport services into the Rutherford CBD from all areas around Maitland as shown in the bus network map below (*Figure 4*). The nearest bus stops are located on the new England Highway along the site frontage Street (see *Photograph 6* below) and on Hillview Street approximately 100 metres north of the site. These are within convenient walking distance of the site ensuring the site is well serviced by public transport (bus) services for use by staff and patients. No additional public transport services or facilities are considered warranted resulting from this development.

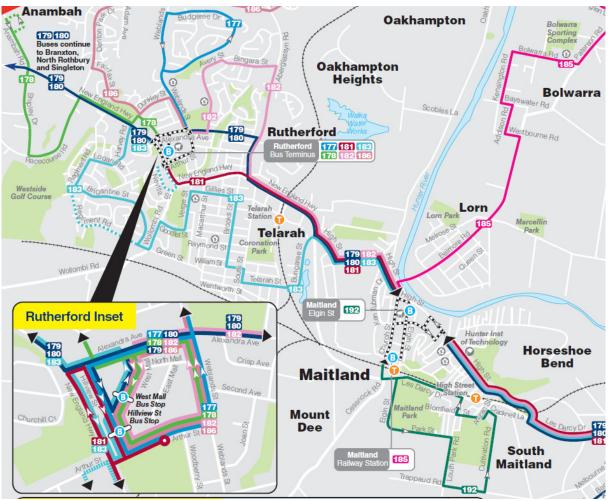


Figure 3 – Hunter Valley Buses routes – Rutherford

Pedestrian and cycle facilities around the site are also considered excellent with suitable concrete and or asphalt / paver footpaths provided within the New England Highway and Rutherford CBD linking to nearby bus stops and the CBD area. A shared pathway for pedestrians and cyclists runs along the eastern side of the New England Highway near the site. Pedestrian crossing facilities / phases are provided within the signalised intersection of the New England Highway and Arthur Street as well as marked foot crossings within the Rutherford CBD area. **Photographs 6 & 7** below show pedestrian and bicycle infrastructure around the site. Overall it is considered the development is unlikely to generate any significant additional pedestrian or bicycle traffic therefore no nexus exists for the provision of additional infrastructure. The existing pedestrian facilities around the site are also considered suitable for the level of demand generated by the development.





Photograph 6 – Bus Stop and Footpaths – New England Highway – site frontage



Photograph 7 – Pedestrian crossing phases New England Highway at Arthur Street



3. CONCLUSIONS

This traffic and parking assessment for alterations and additions to an existing medical centre known as the Lower Hunter Medical Centre on Lots 1 & 2 Section 2 in DP 21143, 269 – 271 New England Highway, Rutherford has concluded:

- The local and state road network have sufficient available two way mid-block capacity to cater for the development without the need to further upgrade the local road network.
- The development is likely to generate an additional approximately 36 vtph during the AM peak traffic period and 30 vtph during the PM peak traffic period.
- The additional traffic on the local and state road network will not adversely impact on the two-way mid-block efficiency of the local and state road network.
- Sidra Intersection modelling has shown the proposed development will not adversely impact on the operation of the New England Highway / Arthur Street signalised cross intersection. Therefore, the proposal will not adversely impact on the efficiency and effectiveness of the local road network.
- The proposed modified vehicular access (combined entry and exit) driveway off Arthur Street for the development is compliant with the requirements of Australian Standard AS2890.1-2004 Parking facilities Part 1 Off street car parking facilities in regard to width and sight lines. The access will continue to operate with uninterrupted flow conditions and will be suitable and safe for use by all vehicles accessing the site.
- ◆ The proposed on-site car parking supply and servicing facilities are deemed to comply with Maitland City Council's DCP (2010), Australian Standard AS2890.1-2004 Parking facilities – Part 1 – Off street car parking facilities and the RTA's Guide to Traffic Generating Developments (2002).
- The development will not generate any significant additional pedestrian and bicycle traffic therefore no nexus exists for the provision of additional pedestrian and bicycle facilities. The existing pedestrian and bicycle travel infrastructure around the site is considered suitable to cater for any minor increase in pedestrian and bicycle traffic resulting from the development.
- Hunter Valley buses already provides an excellent public transport (bus) service around the site and the additional public transport demand resulting from the development would not be considered sufficient to require additional services or facilities. Therefore no nexus exist for the provision of additional services and infrastructure resulting from the development as the existing services and infrastructure is suitable for the development.

4. RECOMMENDATION

Having carried out this traffic and parking assessment for the alterations and additions to an existing medical centre known as the Lower Hunter Medical Centre on Lots 1 & 2 Section 2 in DP 21143, 269 – 271 New England Highway, Rutherford it is recommended that the proposal can be supported as it is considered that it would not adversely impact on the local and state road network and could meet all the requirements of Maitland City Council, TfNSW and Australian Standards.

J.R. Garry BE (Civil), Masters of Traffic

Director

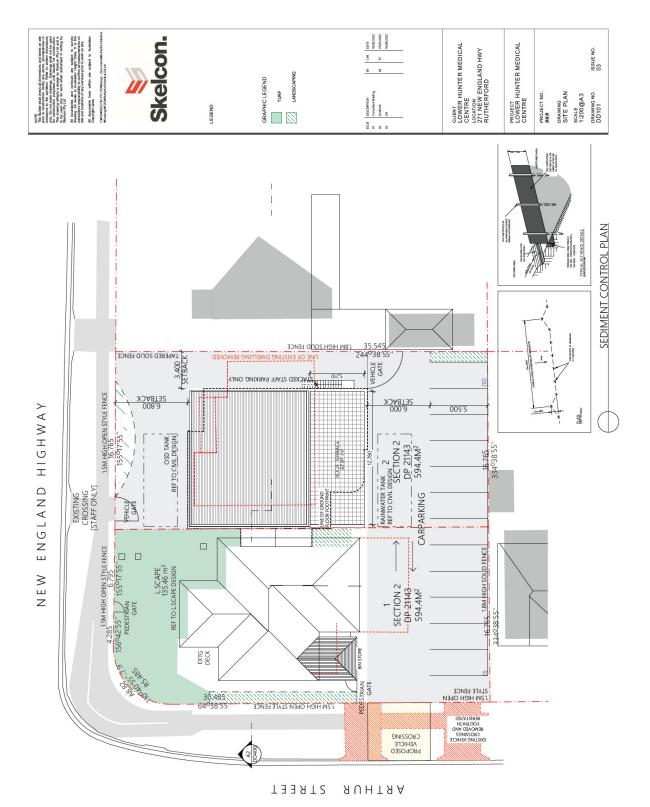
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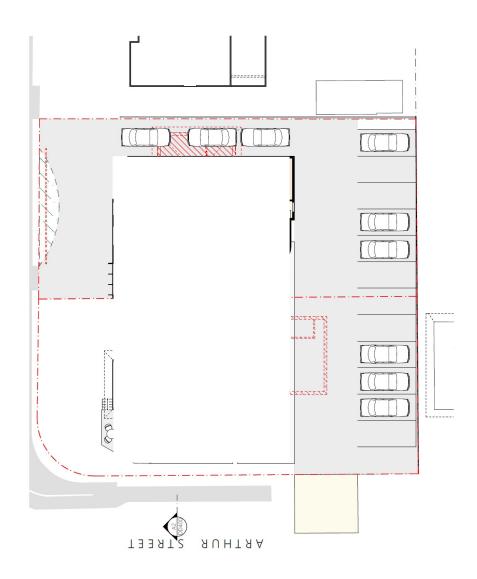


ATTACHMENT A DEVELOPMENT PLANS

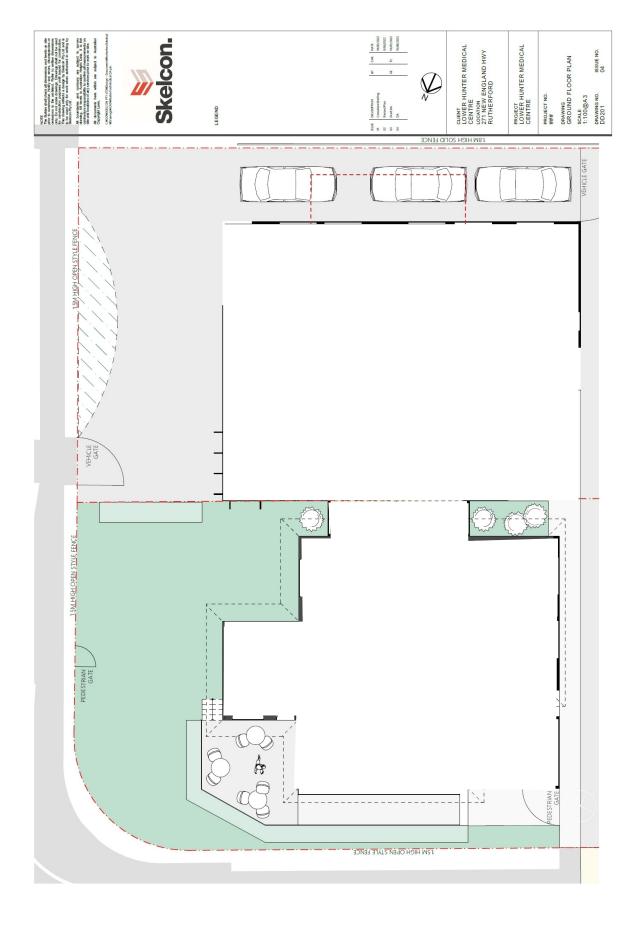




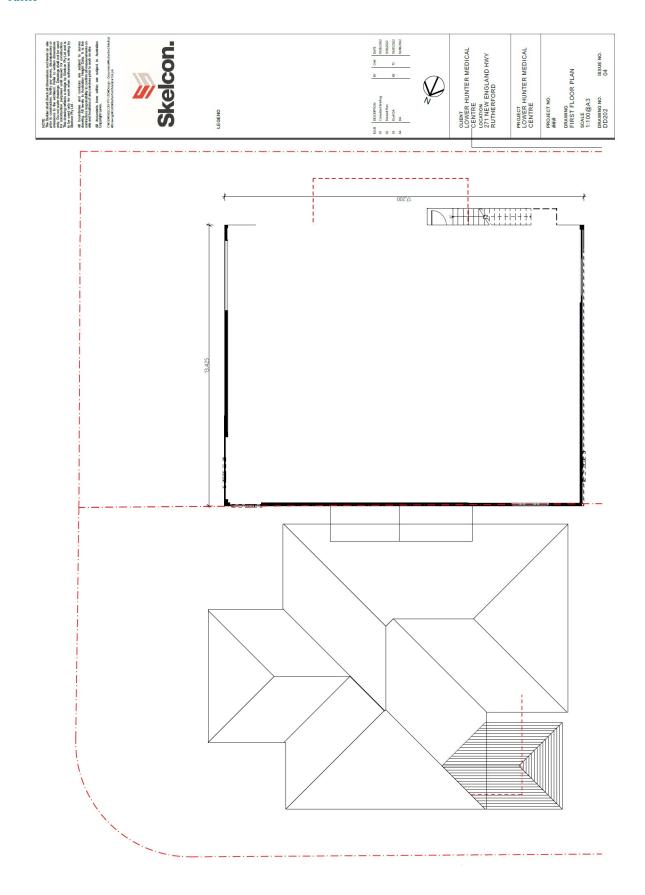
Attachment A



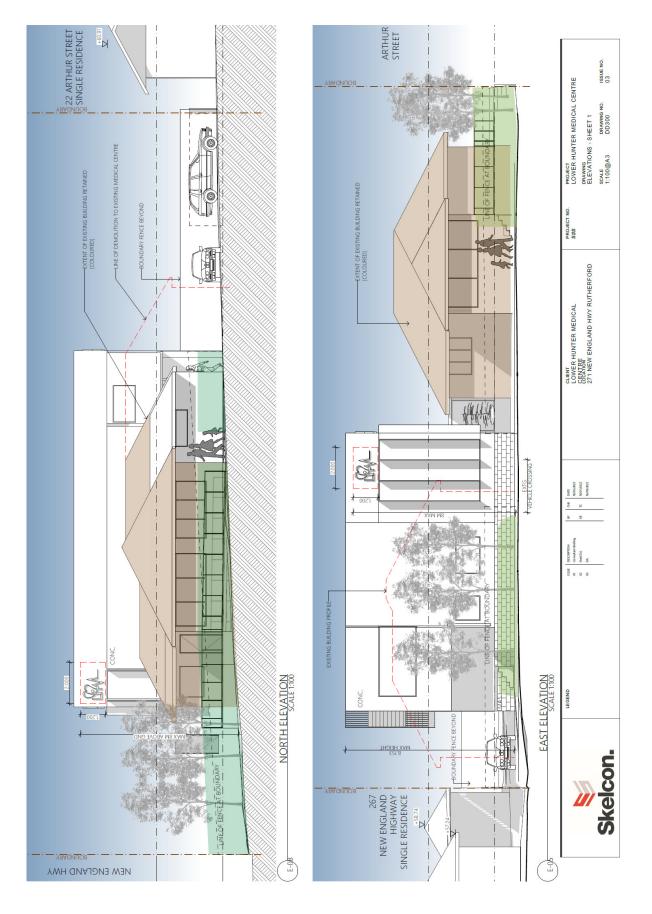




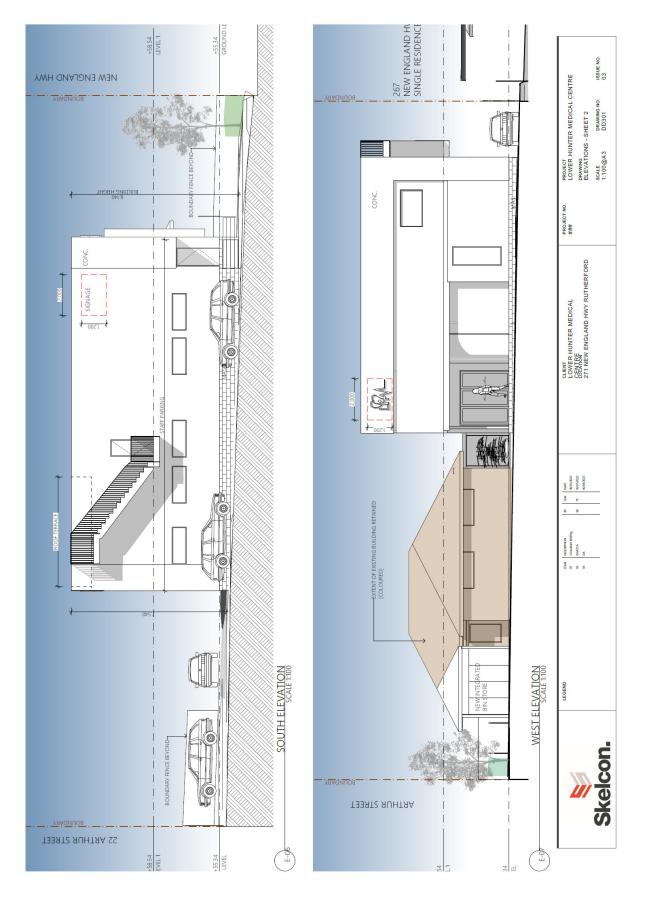






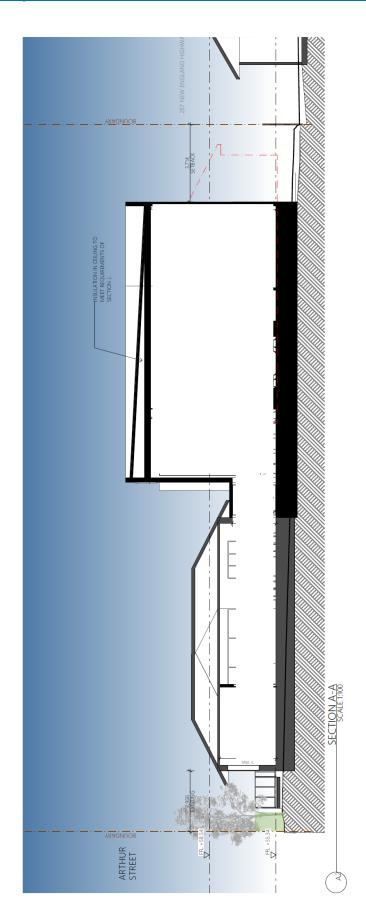










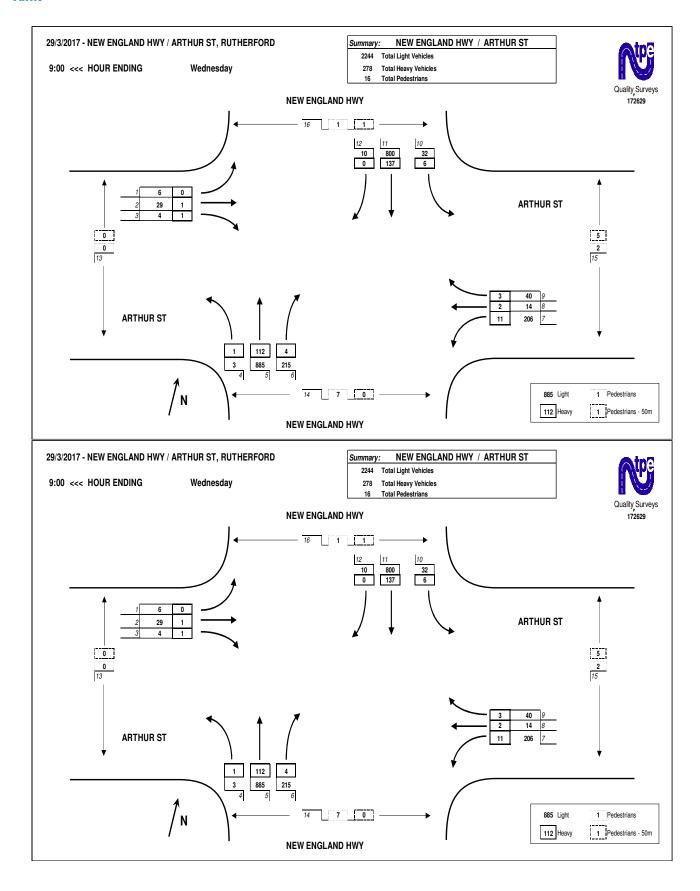






ATTACHMENT B TRAFFIC COUNT TALLY SHEETS







ATTACHMENT C SIDRA MOVEMENT SUMMARY TABLES



Site: 101 [2022AM (Site Folder: General)]

New England Highway / Arthur Street signalised intersection

2017 count + 1.5 % p.a. background traffic growth

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 75 seconds (Site Optimum Cycle Time - Minimum

Delay)

Veh	icle M	ovemen	t Perfo	mance										
Mov ID	Turn	INP VOLU	IMES	DEM FLO	WS	Deg. Satn		Level of Service	QUI	ACK OF EUE	Prop. I Que	Effective Stop	Aver. No.	Aver. Speed
		[Total veh/h	HV] veh/h	[Total veh/h	HV] %	v/c	sec		[Veh. veh	Dist] m		Rate	Cycles	km/h
Sout	th: New	/ England	Highwa	у										
1	L2	4	1	4	25.0	0.006	19.5	LOS B	0.1	8.0	0.60	0.63	0.60	41.8
2	T1	1074	121	1131	11.3	0.754	22.0	LOS B	18.7	143.7	0.91	0.85	0.97	41.2
3	R2	236	4	248	1.7	* 0.781	42.1	LOS C	9.7	68.8	1.00	0.91	1.21	27.4
Appr	roach	1314	126	1383	9.6	0.781	25.6	LOS B	18.7	143.7	0.93	0.86	1.01	38.4
East	: Arthu	Street												
4	L2	234	12	246	5.1	*0.793	41.7	LOS C	9.7	71.1	1.00	0.94	1.23	27.0
5	T1	17	2	18	11.8	0.250	30.1	LOS C	2.2	16.4	0.90	0.73	0.90	28.5
6	R2	46	3	48	6.5	0.250	34.7	LOS C	2.2	16.4	0.90	0.73	0.90	23.6
Appr	roach	297	17	313	5.7	0.793	40.0	LOS C	9.7	71.1	0.98	0.89	1.16	26.7
Nort	h: New	England	Highway	y										
7	L2	40	6	42	15.0	0.757	28.0	LOS B	18.5	145.6	0.92	0.86	0.97	29.3
8	T1	1010	148	1063	14.7	*0.757	22.3	LOS B	18.5	145.6	0.91	0.86	0.97	40.9
9	R2	11	0	12	0.0	0.036	33.7	LOS C	0.4	2.5	0.86	0.67	0.86	32.5
Appr	roach	1061	154	1117	14.5	0.757	22.6	LOS B	18.5	145.6	0.91	0.85	0.97	40.5
Wes	t: Arthu	r Street												
10	L2	6	0	6	0.0	0.178	36.0	LOS C	1.5	11.0	0.91	0.69	0.91	32.8
11	T1	32	1	34	3.1	0.178	31.4	LOS C	1.5	11.0	0.91	0.69	0.91	29.0
12	R2	5	1	5	20.0	0.178	36.1	LOS C	1.5	11.0	0.91	0.69	0.91	35.9
Appr	roach	43	2	45	4.7	0.178	32.6	LOS C	1.5	11.0	0.91	0.69	0.91	30.5
All Vehi	cles	2715	299	2858	11.0	0.793	26.1	LOS B	18.7	145.6	0.93	0.86	1.01	37.7

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Geometric Delay is included).

Queue Model: SIDRA Standard.

Gap-Acceptance Capacity: SIDRA Standard (Akcelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Pedestrian I	Movem	ent Peri	orman	ce							
Mov ID Crossing	Input Vol.	Dem. Flow	Aver. Delay	Level of Service	AVERAGE QUE [Ped		Prop. Et Que	fective Stop Rate	Travel Time		Aver. Speed
	ped/h	ped/h	sec		ped	m			sec	m	m/sec
South: New E	ngland H	Highway									
P1 Full	9	9	31.8	LOS D	0.0	0.0	0.92	0.92	203.9	223.8	1.10
East: Arthur S	treet										
P2 Full	8	8	31.8	LOS D	0.0	0.0	0.92	0.92	196.3	213.9	1.09



Site: 101 [2022PM (Site Folder: General)]

New England Highway / Arthur Street signalised intersection

2017 count + 1.5 % p.a. background traffic growth

Site Category: (None)

Delay)

				mance										
	Turn	INP		DEM		Deg.		Level of		ACK OF		Effective	Aver.	Aver.
ID		VOLU [Total	MES HV1	FLO [Total	WS HV]	Satn	Delay	Service	(QUI	EUE Dist]	Que	Stop Rate	No. Cycles	Speed
		veh/h	veh/h	veh/h	%	v/c	sec		veh	m		rato	0,000	km/h
South	h: New	/ England	Highwa	y										
1	L2	9	1	9	11.1	0.014	28.5	LOS B	0.3	2.5	0.62	0.65	0.62	38.0
2	T1	970	70	1021	7.2	0.707	31.9	LOS C	25.2	187.1	0.89	0.79	0.89	36.1
3	R2	386	18	406	4.7	* 0.935	76.0	LOS F	29.4	214.4	1.00	1.02	1.37	19.3
Appr	oach	1365	89	1437	6.5	0.935	44.3	LOS D	29.4	214.4	0.92	0.85	1.02	30.0
East:	Arthur	Street												
4	L2	334	14	352	4.2	* 0.953	82.3	LOS F	26.4	191.5	1.00	1.07	1.46	18.1
5	T1	28	0	29	0.0	0.358	45.1	LOS D	6.5	46.1	0.91	0.77	0.91	23.9
6	R2	92	3	97	3.3	0.358	49.7	LOS D	6.5	46.1	0.91	0.77	0.91	19.2
Appr	oach	454	17	478	3.7	0.953	73.4	LOSF	26.4	191.5	0.98	0.99	1.31	18.5
North	n: New	England	Highway	/										
7	L2	66	4	69	6.1	0.924	60.9	LOS E	47.7	350.1	1.00	1.08	1.23	17.6
8	T1	1236	70	1301	5.7	*0.924	55.2	LOS D	47.7	350.1	1.00	1.08	1.23	27.9
9	R2	20	1	21	5.0	0.049	42.8	LOS D	0.9	6.8	0.80	0.70	0.80	29.5
Appr	oach	1322	75	1392	5.7	0.924	55.3	LOS D	47.7	350.1	1.00	1.08	1.23	27.5
West	: Arthu	ır Street												
10	L2	10	1	11	10.0	0.522	68.0	LOS E	3.2	23.2	1.00	0.75	1.01	23.6
11	T1	25	0	26	0.0	0.522	63.4	LOS E	3.2	23.2	1.00	0.75	1.01	20.3
12	R2	15	1	16	6.7	0.522	67.9	LOS E	3.2	23.2	1.00	0.75	1.01	27.3
Appr	oach	50	2	53	4.0	0.522	65.7	LOS E	3.2	23.2	1.00	0.75	1.01	23.3
All Vehic	cles	3191	183	3359	5.7	0.953	53.4	LOS D	47.7	350.1	0.96	0.96	1.15	27.0

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Geometric Delay is included).

Queue Model: SIDRA Standard.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Pedestrian I	Movem	ent Per	forman	ce							
Mov ID Crossing	Input Vol.	Dem. Flow	Aver. Delay	Level of Service	AVERAGE QUE [Ped		Prop. Et Que	fective Stop Rate	Travel Time	Travel Dist. S	Aver. Speed
	ped/h	ped/h	sec		ped	m ĺ			sec	m	m/sec
South: New E	ngland H	Highway									
P1 Full	9	9	54.2	LOS E	0.0	0.0	0.95	0.95	226.3	223.8	0.99
East: Arthur S	treet										
P2 Full	8	8	54.2	LOS E	0.0	0.0	0.95	0.95	218.7	213.9	0.98



Site: 101 [2022AM + development (Site Folder: General)]

New England Highway / Arthur Street signalised intersection

2017 count + 1.5 % p.a. background traffic growth

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 75 seconds (Site Optimum Cycle Time - Minimum

Delay)

Vehi	icle M	ovemen	t Perfo	mance										
Mov ID	Turn	INP VOLU [Total		DEM/ FLO		Deg. Satn		Level of Service	95% B/ QUI [Veh.	ACK OF EUE Dist]	Prop. Que	Effective Stop Rate	Aver. No. Cycles	Aver. Speed
		veh/h	veh/h	veh/h	%	v/c	sec		veh	m m		Rate	Cycles	km/h
Sout	h: New	England	Highwa	у										
1	L2	11	1	12	9.1	0.016	19.5	LOS B	0.2	1.9	0.61	0.65	0.61	41.9
2	T1	1074	121	1131	11.3	0.757	22.1	LOS B	18.9	144.7	0.91	0.85	0.97	41.1
3	R2	236	4	248	1.7	* 0.781	42.1	LOS C	9.7	68.8	1.00	0.91	1.21	27.4
Appr	oach	1321	126	1391	9.5	0.781	25.7	LOS B	18.9	144.7	0.93	0.86	1.01	38.4
East	Arthur	Street												
4	L2	234	12	246	5.1	*0.793	41.7	LOS C	9.7	71.1	1.00	0.94	1.23	27.0
5	T1	21	2	22	9.5	0.260	30.2	LOS C	2.3	17.4	0.90	0.73	0.90	28.6
6	R2	46	3	48	6.5	0.260	34.8	LOS C	2.3	17.4	0.90	0.73	0.90	23.7
Appr	oach	301	17	317	5.6	0.793	39.9	LOS C	9.7	71.1	0.98	0.89	1.16	26.7
North	n: New	England	Highway	У										
7	L2	40	6	42	15.0	0.760	28.1	LOS B	18.6	146.5	0.92	0.86	0.98	29.2
8	T1	1010	148	1063	14.7	*0.760	22.3	LOS B	18.6	146.5	0.91	0.86	0.97	40.9
9	R2	16	0	17	0.0	0.052	33.9	LOS C	0.5	3.7	0.86	0.69	0.86	32.5
Appr	oach	1066	154	1122	14.4	0.760	22.7	LOS B	18.6	146.5	0.91	0.85	0.97	40.4
West	t: Arthu	r Street												
10	L2	11	0	12	0.0	0.291	38.7	LOS C	2.2	15.8	0.95	0.73	0.95	31.6
11	T1	36	1	38	2.8	0.291	34.2	LOS C	2.2	15.8	0.95	0.73	0.95	27.7
12	R2	12	1	13	8.3	0.291	38.8	LOS C	2.2	15.8	0.95	0.73	0.95	35.0
Appr	oach	59	2	62	3.4	0.291	36.0	LOS C	2.2	15.8	0.95	0.73	0.95	30.2
All Vehic	cles	2747	299	2892	10.9	0.793	26.3	LOS B	18.9	146.5	0.93	0.86	1.01	37.5

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Geometric Delay is included).

Queue Model: SIDRA Standard.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Pedestrian I	Moveme	ent Perf	ormano	ce							
Mov ID Crossing	Input Vol.	Dem. Flow	Aver. Delay	Level of Service	AVERAGE QUE [Ped		Prop. Et Que	fective Stop Rate	Travel Time	Travel Dist.	Aver. Speed
	ped/h	ped/h	sec		ped	m			sec	m	m/sec
South: New E	ngland F	Highway									
P1 Full	9	9	31.8	LOS D	0.0	0.0	0.92	0.92	203.9	223.8	1.10
East: Arthur S	treet										
P2 Full	8	8	31.8	LOS D	0.0	0.0	0.92	0.92	196.3	213.9	1.09



Site: 101 [2022PM + development (Site Folder: General)]

New England Highway / Arthur Street signalised intersection

2017 count + 1.5 % p.a. background traffic growth

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 120 seconds (Site Optimum Cycle Time - Minimum

Delay)

Vehi	icle M	ovemen	t Perfor	mance										
	Turn	INP		DEM		Deg.	Aver. Lo			ACK OF		Effective	Aver.	Aver.
ID		VOLU [Total	JMES HV1	FLO [Total	WS HV]	Satn	Delay S	Service	QUI [Veh.	EUE Dist 1	Que	Stop Rate	No. Cycles	Speed
		veh/h	veh/h	veh/h	%	v/c	sec		veh	m		Nate	Cycles	km/h
Sout	h: New	England	Highwa	y										
1	L2	15	1	16	6.7	0.022	28.5 I	LOS C	0.5	4.0	0.63	0.67	0.63	38.0
2	T1	970	70	1021	7.2	0.712	31.9 I	LOS C	25.1	186.9	0.89	0.79	0.89	36.1
3	R2	386	18	406	4.7	* 0.935	76.0	LOS F	29.4	214.4	1.00	1.02	1.37	19.3
Appr	oach	1371	89	1443	6.5	0.935	44.2 l	LOS D	29.4	214.4	0.92	0.85	1.02	30.1
East	Arthur	Street												
4	L2	334	14	352	4.2	* 0.953	82.5	LOS F	26.4	191.7	1.00	1.08	1.46	18.1
5	T1	31	0	33	0.0	0.367	45.2 l	LOS D	6.6	47.4	0.91	0.77	0.91	23.9
6	R2	92	3	97	3.3	0.367	49.8 I	LOS D	6.6	47.4	0.91	0.77	0.91	19.2
Appr	oach	457	17	481	3.7	0.953	73.4	LOS F	26.4	191.7	0.98	0.99	1.31	18.5
North	n: New	England	Highway	/										
7	L2	66	4	69	6.1	0.927	61.6 I	LOS E	48.1	353.0	1.00	1.08	1.24	17.5
8	T1	1236	70	1301	5.7	*0.927	55.8 I	LOS D	48.1	353.0	1.00	1.09	1.24	27.7
9	R2	24	1	25	4.2	0.058	42.9 I	LOS D	1.1	8.2	0.80	0.71	0.80	29.4
Appr	oach	1326	75	1396	5.7	0.927	55.9 I	LOS D	48.1	353.0	1.00	1.08	1.23	27.3
West	t: Arthu	r Street												
10	L2	14	1	15	7.1	0.722	71.5	LOS F	4.2	30.3	1.00	0.84	1.20	23.0
11	T1	28	0	29	0.0	0.722	66.8 I	LOS E	4.2	30.3	1.00	0.84	1.20	19.6
12	R2	21	1	22	4.8	0.722	71.4	LOS F	4.2	30.3	1.00	0.84	1.20	26.6
Appr	oach	63	2	66	3.2	0.722	69.4 I	LOS E	4.2	30.3	1.00	0.84	1.20	22.9
All Vehic	cles	3217	183	3386	5.7	0.953	53.7 I	LOS D	48.1	353.0	0.96	0.97	1.15	26.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Geometric Delay is included).

Queue Model: SIDRA Standard.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Pedestrian I	Movem	ent Perl	ormano	e							
Mov .	Input	Dem.	Aver.	Level of	AVERAGE	BACK OF	Prop. Ef	fective	Travel	Travel	Aver.
ID Crossing	Vol.	Flow	Delay	Service	QUE	UE	Que	Stop	Time	Dist. S	Speed
					[Ped	Dist]		Rate			
	ped/h	ped/h	sec		ped	m			sec	m	m/sec
South: New E	ngland F	Highway									
P1 Full	9	9	54.2	LOS E	0.0	0.0	0.95	0.95	226.3	223.8	0.99
East: Arthur S	treet										
P2 Full	8	8	54.2	LOS E	0.0	0.0	0.95	0.95	218.7	213.9	0.98



Site: 101 [2032PM (Site Folder: General)]

New England Highway / Arthur Street signalised intersection

2017 count + 1.5 % p.a. background traffic growth

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 150 seconds (Site Practical Cycle Time)

Design Life Analysis (Final Year): Results for 10 years

Veh	icle M	ovemen	t Perfo	rmance										
Mov ID	Turn	INP VOLU [Total		DEM. FLO [Total		Deg. Satn		Level of Service		ACK OF EUE Dist 1	Prop. F Que	Effective Stop Rate	Aver. No. Cycles	Aver. Speed
		veh/h	veh/h	veh/h	%	v/c	sec		veh.	m		Nate	Cycles	km/h
Sout	th: New	/ England	Highwa	y										
1	L2	9	1	11	11.1	0.017	36.0	LOS C	0.5	3.7	0.65	0.66	0.65	35.2
2	T1	970	70	1185	7.2	0.974	86.0	LOS F ¹¹	64.1	476.6	0.96	1.14	1.31	21.5
3	R2	386	18	472	4.7	* 1.136	211.3	LOS F ¹¹	66.1	481.0	1.00	1.29	2.02	8.6
Appr	roach	1365	89	1668	6.5	1.136	121.1	LOS F ¹¹	66.1	481.0	0.97	1.18	1.51	16.0
East	: Arthu	r Street												
4	L2	334	14	408	4.2	* 1.129	205.1	LOS F ¹¹	55.6	402.9	1.00	1.34	2.00	8.8
5	T1	28	0	34	0.0	0.398	55.5	LOS D ¹¹	9.3	66.5	0.91	0.78	0.91	21.5
6	R2	92	3	112	3.3	0.398	60.1	LOS E ¹¹	9.3	66.5	0.91	0.78	0.91	16.9
Appr	roach	454	17	555	3.7	1.129	166.5	LOS F ¹¹	55.6	402.9	0.98	1.19	1.71	9.8
Nort	h: New	England	Highway	y										
7	L2	66	4	81	6.1	1.130	198.1	LOS F ¹¹	111.2	816.6	1.00	1.61	1.92	6.5
8	T1	1236	70	1510	5.7	* 1.130	192.6	LOS F ¹¹	111.2	816.6	1.00	1.63	1.92	11.7
9	R2	20	1	24	5.0	0.049	47.2	LOS D ¹¹	1.3	9.4	0.76	0.70	0.76	28.2
Appr	roach	1322	75	1615	5.7	1.130	190.7	LOS F ¹¹	111.2	816.6	1.00	1.62	1.90	11.6
Wes	t: Arthu	ır Street												
10	L2	10	1	12	10.0	0.666	85.5	LOS F ¹¹	4.7	34.2	1.00	0.81	1.11	20.6
11	T1	25	0	31	0.0	0.666	80.8	LOS F ¹¹	4.7	34.2	1.00	0.81	1.11	17.5
12	R2	15	1	18	6.7	0.666	85.4	4.4	4.7	34.2	1.00	0.81	1.11	24.2
Appr	roach	50	2	61	4.0	0.666	83.1	LOS F ¹¹	4.7	34.2	1.00	0.81	1.11	20.4
All Vehi	cles	3191	183	3898	5.7	1.136	155.8	LOS F ¹¹	111.2	816.6	0.98	1.36	1.69	13.0

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Geometric Delay is included).

Queue Model: SIDRA Standard.

Gap-Acceptance Capacity: SIDRA Standard (Akcelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

- 11 Level of Service is worse than the Level of Service Target specified in the Parameter Settings dialog.
- * Critical Movement (Signal Timing)

Mov	Input	Dem.	Aver.	Level of	AVERAGE	BACK OF	Prop. Et	fective	Travel	Travel	Aver.
ID Crossing	Vol.	Flow	Delay	Service	QUE	UE	Que	Stop	Time	Dist. S	Speed
					[Ped	Dist]		Rate			
	ped/h	ped/h	sec		ped	m			sec	m	m/sec
South: New E	ngland F	Highway									
P1 Full	9	11	69.2	LOS	0.0	0.0	0.96	0.96	241.3	223.8	0.93
				F ¹²							
East: Arthur S	treet										



Site: 101 [2032AM + development (Site Folder: General)]

New England Highway / Arthur Street signalised intersection

2017 count + 1.5 % p.a. background traffic growth

Site Category: (None)

Design Life Analysis (Final Year): Results for 10 years

Vehi	cle M	ovemen	t Perfo	rmance										
Mov ID	Turn	INF VOLU [Total veh/h		DEM FLO [Total veh/h		Deg. Satn v/c		Level of Service		ACK OF EUE Dist] m	Prop. Que	Effective Stop Rate	Aver. No. Cycles	Aver. Speed km/h
South	n: New	/ England			70	V/C	300		VOII	- '''				KIII/II
1	L2	11	1	13	9.1	0.019	20.4	LOS B	0.3	2.3	0.61	0.66	0.61	41.5
2	T1	1074	121	1312	11.3	*0.904	38.5	LOS C	31.6	242.6	0.99	1.12	1.30	33.3
3	R2	236	4	288	1.7	*0.898	53.1	LOS D ¹¹	13.6	96.8	1.00	1.03	1.46	24.1
Appro	oach	1321	126	1614	9.5	0.904	41.0	LOS C	31.6	242.6	0.99	1.10	1.32	31.7
East:	Arthur	Street												
4	L2	234	12	286	5.1	* 0.851	46.7	LOS D ¹¹	12.6	92.3	1.00	0.99	1.32	25.5
5	T1	21	2	26	9.5	0.282	32.0	LOS C	2.9	21.4	0.91	0.74	0.91	27.9
6	R2	46	3	56	6.5	0.282	36.6	LOS C	2.9	21.4	0.91	0.74	0.91	23.0
Appro	oach	301	17	368	5.6	0.851	44.1	LOS D ¹¹	12.6	92.3	0.98	0.93	1.23	25.4
North	: New	England	Highway	y										
7	L2	40	6	49	15.0	0.884	40.8	LOS C	29.1	229.4	0.99	1.08	1.24	23.3
8	T1	1010	148	1234	14.7	0.884	35.1	LOS C	29.1	229.4	0.99	1.08	1.24	34.7
9	R2	16	0	20	0.0	0.060	35.6	LOS C	0.6	4.5	0.86	0.70	0.86	31.8
Appro	oach	1066	154	1302	14.4	0.884	35.3	LOS C	29.1	229.4	0.99	1.07	1.23	34.3
West	: Arthu	ır Street												
10	L2	11	0	13	0.0	0.359	41.9	LOS C	2.7	19.8	0.96	0.75	0.96	30.5
11	T1	36	1	44	2.8	0.359	37.3	LOS C	2.7	19.8	0.96	0.75	0.96	26.7
12	R2	12	1	15	8.3	0.359	42.0	LOS C	2.7	19.8	0.96	0.75	0.96	33.9
Appro	oach	59	2	72	3.4	0.359	39.1	LOS C	2.7	19.8	0.96	0.75	0.96	29.2
All Vehic	les	2747	299	3356	10.9	0.904	39.1	LOSC	31.6	242.6	0.99	1.06	1.27	31.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Geometric Delay is included).

Queue Model: SIDRA Standard.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

11 Level of Service is worse than the Level of Service Target specified in the Parameter Settings dialog.

Pedestrian I	Movem	ent Perí	orman	ce							
Mov ID Crossing	Input Vol.	Dem. Flow	Aver. Delay	Level of Service	AVERAGE QUE [Ped		Prop. Et Que	fective Stop Rate	Travel Time	Travel Dist. 9	Aver. Speed
	ped/h	ped/h	sec		ped	m			sec	m	m/sec
South: New Er	ngland H	Highway									
P1 Full	9	11	34.2	LOS D	0.0	0.0	0.93	0.93	206.4	223.8	1.08
East: Arthur S	treet										
P2 Full	8	10	34.2	LOS D	0.0	0.0	0.93	0.93	198.8	213.9	1.08



Site: 101 [2032PM + development (Site Folder: General)]

New England Highway / Arthur Street signalised intersection

2017 count + 1.5 % p.a. background traffic growth

Site Category: (None)

Design Life Analysis (Final Year): Results for 10 years

Vehicle Movement Performance														
	Mov Turn INPUT ID VOLUMES			DEMAND FLOWS		Deg. Satn	Aver. Level of Delay Service		95% BACK OF QUEUE		Prop. Effective Que Stop		Aver.	Aver. Speed
שו		[Total	HV]	[Total	WS HV]	Saui	Delay	Service	[Veh.	Dist]	Que	Rate	Cycles	Speeu
		veh/h	veh/h	veh/h	%	v/c	sec		veh	m				km/h
Sout	South: New England Highway													
1	L2	15	1	18	6.7	0.027	36.1	LOS C	8.0	6.1	0.65	0.68	0.65	35.2
2	T1	970	70	1185	7.2	0.979	88.4	LOS F ¹¹	64.5	479.6	0.96	1.16	1.33	21.2
3	R2	386	18	472	4.7	*1.137	211.9	LOS F ¹¹	66.2	481.6	1.00	1.29	2.02	8.6
Appr	oach	1371	89	1675	6.5	1.137	122.6	LOS F ¹¹	66.2	481.6	0.97	1.19	1.52	15.9
East: Arthur Street														
4	L2	334	14	408	4.2	*1.129	205.2	LOS F ¹¹	55.6	403.0	1.00	1.34	2.00	8.8
5	T1	31	0	38	0.0	0.412	56.5	LOS D ¹¹	9.6	68.7	0.92	0.78	0.92	21.3
6	R2	92	3	112	3.3	0.412	61.1	LOS E ¹¹	9.6	68.7	0.92	0.78	0.92	16.7
Appr	oach	457	17	558	3.7	1.129	166.1	LOS F ¹¹	55.6	403.0	0.98	1.19	1.71	9.9
North: New England Highway														
7	L2	66	4	81	6.1	1.133	200.3	LOS F ¹¹	112.1	823.0	1.00	1.62	1.93	6.4
8	T1	1236	70	1510	5.7	* 1.133	194.9	LOS F ¹¹	112.1	823.0	1.00	1.64	1.93	11.6
9	R2	24	1	29	4.2	0.058	47.4	LOS D ¹¹	1.5	11.2	0.76	0.71	0.76	28.2
Appr	oach	1326	75	1620	5.7	1.133	192.5	LOS F ¹¹	112.1	823.0	1.00	1.62	1.91	11.5
West: Arthur Street														
10	L2	14	1	17	7.1	0.916	97.7	LOS F ¹¹	6.5	46.8	1.00	1.00	1.52	19.0
11	T1	28	0	34	0.0	0.916	93.1	LOS F ¹¹	6.5	46.8	1.00	1.00	1.52	15.9
12	R2	21	1	26	4.8	0.916	97.7	LOS F ¹¹	6.5	46.8	1.00	1.00	1.52	22.3
Appr	oach	63	2	77	3.2	0.916	95.6	LOS F ¹¹	6.5	46.8	1.00	1.00	1.52	18.9
All Vehic	cles	3217	183	3930	5.7	1.137	157.0	LOS F ¹¹	112.1	823.0	0.98	1.36	1.71	12.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Geometric Delay is included).

Queue Model: SIDRA Standard.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

- 11 Level of Service is worse than the Level of Service Target specified in the Parameter Settings dialog.
- * Critical Movement (Signal Timing)

Pedestrian Movement Performance											
Mov Input Dem. ID Crossing Vol. Flow			Aver. Delay	Level of AVERAGE BACK OF Service QUEUE			Prop. Effective Que Stop		Travel Time	Travel Aver. Dist. Speed	
	ped/h	ped/h	sec		[Ped ped	Dist] m		Rate	sec	m	· m/sec
South: New England Highway											
P1 Full	9	11	69.2	LOS F ¹²	0.0	0.0	0.96	0.96	241.3	223.8	0.93
East: Arthur S	treet										